

Traffic Impact Study

Prepared For Polar Views, LLC

Located at

39 Lamartine Street Worcester, MA



August 2024

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1 INTRODUCTION

Polar Views, LLC, hereafter referred to as the applicant, is proposing the development of a parcel of land totaling 18,154 square feet (sf) to construct a six-story building housing a total of 36 apartment units and 1,581-sf of first floor retail. The proposed development is located behind 25 Meade Street, the Department of Inspectional Services offices in Worcester, Massachusetts. The applicant is proposing to evaluate the impact of this development on area roadway traffic and consider any improvements that may be necessary to make this development feasible and acceptable. This traffic study is prepared to make this evaluation. The purpose of this traffic study is to develop an understanding of existing traffic operations and concerns, forecast future site generated traffic, assess the adequacy of the existing roadway system to accommodate the proposed development into the future, and to identify and recommend appropriate mitigation strategies, should any be deemed necessary.

Project Description

The applicant proposes to develop an 18,154-sf parcel of land and construct a total of 36 apartment units and a 1,581-sf retail space on the first floor. This parcel of land is currently vacant with no structures on it. All units will be accessed via an underground entrance driveway from Meade Street and an at-grade garage entrance from Grosvenor Street. These driveways will provide access to all 40 proposed off-street parking spaces. The Meade Street driveway is 24 feet wide while the Grosvenor Street driveway is 28 feet wide, both of which are designed to provide two-way traffic circulation. Of the 40 parking spaces, 13 will be underground and the remaining 27 parking spaces will be at grade. Also, three parking spaces will be designed and designated as handicap parking spaces and strategically located near elevators and stairways within the parking areas. Finally, six of the ground level parking spaces will be equipped with Electric Vehicle (EV) charging devices.

The proposed site is in the Business, General (BG-3.0) zoning district and is currently vacant as its approximate location is shown in the aerial photograph in Figure 1.



Figure 1 - Proposed Apartment Building and Retail Development Site

2 EXISTING CONDITIONS

Evaluation of the transportation impacts associated with the proposed mixed-use development project requires an understanding of the existing transportation system in the immediate vicinity of the site. Evaluating the existing study area streets operating conditions necessitates an examination of existing roadway traffic volumes, geometric features, and local community traffic-related issues. Each of these elements is described below.

Study Area Roadway Network

In consultation with the city of Worcester Department of Transportation and Mobility (DTM), the study area for this traffic impact report has been defined to include the evaluation of the following intersections.

- Intersection of Lafayette Street and Grosvenor Street
- Intersection of Lafayette Street and Meade Street
- Intersection of Meade Street and Lamartine Street
- Intersection of Grosvenor Street and Lamartine Street
- Intersection of Lamartine Street, Lodi Street, Green Island Boulevard and Hermon Street

Additionally, per Worcester DTM, a total of eight Automatic Traffic Recorders (ATRs) were placed at the following locations.

- Across Hermon Street
- Across Lodi Street
- Across Lamartine Street east of Lodi Street
- Across Lamartine Street west of Lodi Street
- Across Green Island Boulevard
- Across Grosvenor Street
- Across Meade Street
- Across Lafayette Street

Lamartine Street is a two-way street with one travel lane in each direction. It intersects Quinsigamond Avenue at its terminus in a southwesterly direction and it intersects Millbury Street at its easterly end. It is flat and straight in the vicinity of the proposed site. The segment of Lamartine Street located between Quinsigamond Avenue and Green Island Boulevard that was recently reconstructed is 40 feet in width and it features bike lanes in both directions and on-street parking on the northeasterly side of the street. It is striped with double yellow center lines and its southwesterly approach at Quinsigamond Avenue is controlled by a fully actuated traffic signal system and its northeasterly approach at Green Island Boulevard is controlled by a stop sign. The section of Lamartine Street between Green Island Boulevard and Millbury

Street is 28 feet wide and was also recently reconstructed with new sidewalks and Americans with Disability Act (ADA) compliant handicap ramps. The north side of this section of Lamartine Street is posted with temporary no parking anytime signs, possibly for construction reasons. The approaches of this segment are also controlled by stop signs at Green Island Boulevard, Washington Street, Harding Street, and Millbury Street. This section is also striped with double yellow center lines. Finally, there are crosswalk markings across Lamartine Street at all its above-mentioned intersections. Land use along Lamartine Street is a combination of commercial and industrial uses.

Meade Street is a two-way street and traverses in the northerly and southerly directions. It has one lane of travel in each direction and its pavement width is 28 feet. It provides sidewalks on both sides of the street. It is 590 feet long and it intersects Lamartine Street to the north forming a "T" intersection and Lafayette Street to the south forming another "T" intersection. The land use on the southern half of the street is residential, consisting mostly of double and triple-deckers. The northerly half consists of the Worcester Department of Inspectional Services building on the west side and a parking lot serving that department as well as for Polar Park events on the east side. Presently, there are no on-street parking restrictions on Meade Street to accommodate the residential uses, except a dedicated handicap parking space in front of number 12 Meade Street, a 100-foot No Parking Anytime (NPA) on the west side of Meade Street from a point just north of the Worcester Inspectional Services Department building entrance in the northerly direction, and a distance of 250 feet NPA on the east side of the street from Lamartine Street southerly.

Grosvenor Street is also a two-way street with a pavement width of 28 feet. It also has sidewalks on both sides of the street. It runs parallel to Meade Street and is the same length at 590 feet. It intersects Lamartine Street to the north forming a "T" intersection and Lafayette Street to the south forming another "T" intersection. The land use consists of entirely residential double and tripledeckers, except the portion that abuts the Department of Inspectional Services. Presently, there are no on-street parking restrictions on Grosvenor Street to accommodate the residential uses, except for a dedicated handicap parking space in front of number 31 Grosvenor Street and one in front of number 46 Grosvenor Street.

Intersection of Meade Street and Lamartine Street is a three-legged intersection, and its northbound approach is controlled by a stop sign, although it appears that the stop sign is missing. Its approaches are 14 feet wide except for the northbound approach that widens to 23 feet at the stop bar. There is a crosswalk across the northbound approach of Meade Street that is supplemented with a stop bar. This crosswalk is equipped with HP ramps that include tactile surfaces.

Intersection of Meade Street and Lafayette Street is also a three-legged intersection, and its southbound approach is controlled by a stop sign. Its approaches are 14 feet in width and there is a crosswalk spanning across Meade Street that also has a stop bar. Again, this crosswalk is equipped with HP ramps that include tactile surfaces.

Intersection of Lamartine Street, Green Island Boulevard, Hermon Street and Lodi Street is a five-legged intersection located approximately 200 feet west of the Lamartine and Grosvenor Streets intersection. Of the five streets diverging at this intersection, Lodi Street is a one-way street traversing in the southerly direction, thus, for all intents and purposes, it operates as a four-legged intersection. Its northeasterly and southwesterly approaches are 16 feet in width and feature a bike lane. The Lamartine Street approach is 14 feet, while the Hermon Street approach is 12 feet wide in addition to a 2-foot shoulder. All legs of this intersection are marked with crosswalks and associated HP ramps that include tactile surfaces. All legs of this intersection except the Lodi Street leg which is a one-way street in the southerly direction are controlled with stop signs and are marked with stop bars.

Intersection of Grosvenor and Lamartine Streets is a three-legged "T" intersection and its northbound approach, which is 14 feet wide, is controlled with a stop sign but the stop sign is missing. As stated herein above, Lamartine Street is a two-way street and on-street parking is allowed on the north side of the street. There is a crosswalk across Grosvenor Street supplemented with a stop bar.

Intersection of Grosvenor and Lafayette Streets is also a three legged "T" intersection and its southbound approach, which is 14 feet wide, is controlled by a stop sign, again it appears that the stop sign is missing. Lafayette Street is a one-way street in the easterly direction and its easterly Street approach is 15 feet wide with parking allowed on both sides of the street. There is a crosswalk across Grosvenor Street.

Traffic Volumes

New peak hour turning movement counts (TMCs) were collected on Wednesday July 17th and Thursday July 18th, during two-hour periods between the hours of 7-9 AM and 4-6 PM commuter peak periods in order to identify the critical peak hour. The following Figure 2 shows the baseline peak hour traffic counts conducted on the above dates for all five intersections. This standard practice, which requires the evaluation of the intersections during critical peak traffic, is intended to help determine the traffic impacts of the proposed residential development on the area streets and their intersections under worst case scenario. Table 1 below shows the daily traffic, AM and PM peak hour traffic, average (Mean Speed), and 85th percentile speed at eight locations within the study area.

Street	Lamartine St E	Lamartine St S	Meade St	Green Island St	Grosvenor St	Hermon St	Lafayette St	Lodi St
ADT	3,902	3,271	129	2,472	235	1,111	889	289
AM Peak Vol	268	146	21	197	18	134	61	28
PM Peak Vol	310	256	16	218	32	185	65	33
Average Speed mph	20.6	29.5	13.5	21.8	15.6	21.5	19.2	15.2
85th% Speed mph	24	34	18	24	20	25	20	16

Table 1 Automatic Traffic Recorders Results



Figure 2 – Baseline Turning Movement Counts

The COVID-19 pandemic had caused a drop in vehicular traffic over the last few years. In April 2020, *massDOT* published the Guidance on Traffic Count Data and how to estimate existing and future traffic for counts taken after March 13, 2020. The procedure to adjust the TMCs to pre-COVID conditions requires the use of historical and seasonal data provided by the *massDOT*, to adjust for seasonal and historical changes, and then forecast the data to the existing year. The *massDOT* has provided updated guidance that no longer requires pandemic-related adjustment of traffic counts performed after March 2022 except in locations where the predominant land use consists of offices or other similar uses. Therefore, given the predominant land use in the study area is not of office use, traffic volumes traveling through these intersections were not adjusted.

The *massDOT* provides the latest (2019) seasonal adjustment factors. Based on the *massDOT* Traffic Volume and Classification, the streets within the study area are included in the group U4-U7 for Growth Factor and Seasonal Factor. To evaluate the potential for seasonal fluctuation of traffic volumes on roadways near the proposed site, weekday seasonal factors were obtained from the *massDOT* Statewide Traffic Data Collection. The review of the *massDOT* seasonal adjustment factors shows that roadways having similar characteristics to roadways within the study area (U4-U7) have a factor of 0.92 for traffic counts collected in the month of July. However, because this factor results in a reduction in the volumes, no adjustments were made in order to assess the worst-case scenario. Therefore, the TMCs were not adjusted by this factor. A copy of the *massDOT* seasonal adjustment factors is included in the Technical Appendix section of this

report. The baseline (year 2024) traffic volumes for the AM and PM peak periods are shown in Figure 2 above.

Typically, the PM peak period has the higher volumes, and is considered the critical peak. As is the case here, higher traffic volumes through the study area intersections also occur during the PM peak period.

Safety Concerns

Sight Distances: To evaluate the safety of traffic to and from the site via its driveways, sight distances were measured in the field and analyzed.

Sight distance is defined in the *massDOT* Project Development and Design Guide as the length of roadway ahead that is visible to road users. In most cases, specific sight distance measures apply to motor vehicles and bicyclists. There are two aspects of sight distance that apply to this case. They are:

- Stopping sight distance
- Intersection sight distance

The sight distances are related to the design speed (posted speed) of the roadway and are based on the standards of the American Association of State Highway and Transportation Officials (AASHTO) *A Policy on Geometric Design of Highways and Streets,* also referred to as the *Green Book.*

Stopping Sight Distance is further described in the *massDOT* Project Development and Design Guide as the distance necessary for a vehicle traveling at the design speed (posted speed) before reaching a stationary object in its path. The sight distance at every point along a roadway should be at least the stopping sight distance.

The sight distance for vehicles leaving the site via the Meade Street access driveway to the left (north) were measured in the field. The measured distance is from a point five feet back of a stop bar (approximately 15 feet from the street line) and 3.5 feet above grade to represent drivers' eye height to an object 3.5 feet above roadway grade. The field review of Meade Street showed that the available sight line to the left (north) for vehicles exiting the site driveway is 115 feet all the way to its intersection with Lamartine Street. The field review of Meade Street showed that the available sight line to the right (south) for vehicles exiting the site driveway is 390 feet. As stated earlier, there are no posted speed limit signs on Meade Street. Therefore, the statutory prima facie speed limit of 30 miles per hour applies to this street. Similarly, the sight distance for vehicles leaving the site via the Grosvenor Street access driveway was measured in the field. The field review of Grosvenor Street showed that the available sight line to the left (south) for vehicles exiting the site driveway is 250 feet. The field review of Grosvenor Street showed that the available sight line to the right (north) for vehicles exiting the site driveway is 125 feet all the way to its intersection with Lamartine Street. As stated earlier, there are no posted speed limit signs on Grosvenor Street. Therefore, the statutory prima facie speed limit of 30 miles per hour also applies to this street. The available sight distances for the site driveways are also shown visually in the following photographs.

Based on Basic Design Controls for roadway design, the Stopping Sight Distance is calculated using the formula $d=(V^*V)/(30^*f)$, plus the time required for perception and reaction by a driver (2.5 seconds). V is approach speed in mph, and f=0.28-0.35. The stopping sight distances are calculated and are provided in Exhibit 3-8 of the 2006 *massDOT* Project Development and Design Guide. A copy of this exhibit is presented in the Technical Appendix section of this report. Due to the less than 1% grade of either street, the required stopping sight distance for 30 mph is 200 feet for points south of the proposed site for both streets. The sight distance was also examined vertically. The following aerial photos in Figures 3 & 4 illustrate the profile of both streets in the vicinity of the proposed driveway.

It should be noted that the proposed driveways are 115-125 feet from the Lamartine Street intersections. Therefore, vehicles either arriving at (northbound) or approaching from (southbound) Lamartine Street must make a right or left turn maneuver at much lower speeds than the speed limit or from a stop. As a result, vehicles moving in either direction from all three streets are not expected to reach speeds higher than 10 mph. Due to the less than 1% grade of either street, the required stopping sight distance for 10 mph is less than 100 feet. As demonstrated herein above, the available sight distances are greater than the standard values for vehicles traveling at 10 mph passing the site driveways. Therefore, proper sight distances will be provided.

Intersection Sight Distance is explained by the *massDOT* Project Development and Design Guide as a sight distance at an intersection to allow drivers to perceive the presence of potentially conflicting vehicles. This should occur in sufficient time for a motorist to stop or adjust their speed, as appropriate, to avoid colliding in the intersection. Intersection sight distance also allows drivers of stopped vehicles with a sufficient view of the intersecting roadway to decide when to enter or cross the intersecting roadway. The AASHTO *Green Book* provides procedures to determine desirable intersection sight distances at intersections for various cases, one of which is Intersection Sight Triangle. Exhibit 3-11 of the *massDOT* Project Development and Design Guide demonstrates the sight distances needed based on Intersection Sight Triangle methodology. A copy of Exhibit 3-11 is included in the Technical Appendix section of this report. As shown in this exhibit, there are no values for such low speeds.

Again, it should be noted that vehicles turning from or approaching Lamartine Street must slow down to speeds lower than 10 mph or stop. It also should be noted that there are several nearby driveways at the same distance from Lamartine Street that serve large surface parking lots that generate much more traffic than the proposed site will. Two examples include Canal District Parking Lot at 156 Washington Street, and another located at 163 Washington Street.

In conclusion, driveways in an urban setting like the proposed site are not subjected to follow such standards specifically since these streets are short in length and on-street parking is allowed, thus not affording motorists the opportunity to reach the speed limit. As demonstrated in Table 1, the 85th percentile speed for these streets ranges between 18-24 mph.



Figure 3 – Meade Street profile at proposed driveway



Figure 4 - Grosvenor Street profile at proposed driveway

39 Lamartine Street Worcester, MA

Views from proposed Meade Street driveway looking to the left (north) and right (south)



Views from proposed Grosvenor Street driveway looking to the right (north) and left (south)



Accidents: The latest accident data compiled by the *massDOT* were obtained and reviewed for a five-year period of January 1, 2019-December 31, 2023. This review summarizes the total number of accidents that occurred at each of the five intersections within the study area during this period, and the summary is listed in Table 2, below. It is noted that a total of three accidents occurred during this period at the intersection of Lamartine Street, Hermon Street, Lodi Street and Green Island Boulevard, while a total of 2 accidents were reported during the same period for the intersection of Lafayette Street and Meade Street. None of the accidents at these two intersections involved injuries. Also, none of the accidents took place during peak traffic periods. The breakdown of all accidents at these two intersections is also presented below in Table 2. Finally, the remaining three intersections had no accidents reported during this five-year period.

Using the baseline turning movement counts compiled during traffic surveys of these five intersections, accident rates were calculated in accidents per million vehicles entering each intersection. Utilizing the *massDOT* prescribed methodology, the accident rates for these intersections were calculated and compared with the *massDOT*'s latest available rate of 0.61 for unsignalized intersections on roadways in District 3 of the *massDOT*, in which the City of Worcester

is located. A summary of the accident rates is also included in the following Table 2. A copy of the accident rate calculation sheets is included in the Technical Appendix section of this report. Also included in the Technical Appendix section of this report is a copy of the *massDOT* Average Crash Rates for signalized and unsignalized intersections throughout the Commonwealth of Massachusetts.

The above accident analysis indicates that the rate of accidents for the intersection of Lamartine Street, Hermon Street, Lodi Street and Green Island Boulevard is significantly lower than the *massDOT* rate of 0.61. However, the accident rate for the intersection of Lafayette Street and Meade Street is much higher than the *massDOT* average. Both accidents at this location were of angle type, suggesting potentially lack of sufficient sightline for vehicles making a left-turn maneuver from Meade Street onto Lafayette Street traffic possibly due to parked vehicles on the north side of Lafayette Street.

	Lamartine Meade St	Lamartine Grosvenor	Lamartine Green Island	Lfayette Grosvenor	Lafayette Meade
Intersection	Unsignalized	Unsignalized	Unsignalized	Unsignalized	Unsignalized
Calculated Crash	Rate 0	0 massDOT Av Rate	0.35 0.61	0	1.32
		Year			
2019	0	0	1	0	0
2020	0	0	1	0	1
2021	0	0	0	0	0
2022	0	0	0	0	1
2023	0	0	1	0	0
Total	0	0	3	0	2
		Collision	Туре		
Angle	0	0	2	0	2
Head-On	0	0	0	0	0
Rear-end	0	0	0	0	0
Sideswipe	0	0	0	0	0
Single Vehicle	0	0	1	0	0
Unknown	0	0	0	0	0
Total	0	0	3	0	2
		Severi	ty		
Fatal Injury	0	0	0	0	0
Non-Fatal Injur	y 0	0	0	0	0
Property Dama	ge 0	0	3	0	2
Total	0	0	3	0	2
		Time of	Day		
7:00 AM to 9:0	0 AM 0	0	0	0	0
4:00 PM to 6:00	0 PM 0	0	0	0	0
Other Times	0	0	3	0	2
Total	0	0	3	0	2
		Pavement Co	onditions		
Dry	0	0	1	0	2
Wet	0	0	1	0	0
Snow/Ice	0	0	1	0	0
Total	0	0	3	0	2

Table 2 - Vehicle Crash Summary for 5 Years (Jan 1, 2019-Dec 31, 2023)

Source: massDOT Crash Portal Jan 1, 2023-Dec 31, 2023

Existing Conditions Summary

The proposed site consists of a vacant parcel of land located next to the Worcester Inspectional Services Department at 25 Meade Street. The applicant proposes to build a six-story building to house a total of 36 apartment units and a total of 40 off-street parking spaces, as well as 1,581 sf of retail space on the first floor of the proposed building. Of the 40 parking spaces, three will be designed and designated as van accessible handicap parking spaces. Also, six of the parking spaces will be for Electric Vehicles (EV) and equipped with charging devices. Finally, secure and weather protected bicycle parking is proposed for nine bicycles, as well as exterior bicycle racks which will accommodate 10 bicycles.

Lamartine Street can be characterized as a two-lane roadway with one travel lane in each direction along its entire length. The segment of Lamartine Street between Quinsigamond Avenue and Green Island Boulevard also provides bike lanes as well as on-street parking on one side of the street. The segment between Green Island Boulevard and Millbury Street, that the proposed site fronts on, is 28 feet wide and is posted with No Parking Anytime (NPA) signs on the south side and temporary NPA signs on the north side of the street. Sidewalks are provided on both sides of the entire length of Lamartine Street. It is a local street that serves many types of land uses, including industrial, office and residential properties.

The current land use designation for the site of the proposed development project is Business, General (BG-30), and is currently vacant.

3 **FUTURE CONDITIONS**

Where possible, traffic volumes in the study area are projected to post-development levels. Projected traffic volumes include the existing traffic data obtained from the turning movement counts adjusted to represent the baseline, projected into the future year 2029 peak hours to reflect increases due to future area projects or background growth, and added to the new traffic expected to be generated by the proposed residential development site.

Site-Generated Traffic

The magnitude of traffic volumes that will be generated by the proposed residential and retail development site was projected by using the latest Trip Generation Manual published by the Institute of Transportation Engineers (ITE).

Based on the ITE Trip Generation Manual, the rates at which the proposed land uses generate traffic vary depending upon the time of day. These rates were used to calculate the number of trips expected to be generated by the proposed residential and retail development during an average weekday morning and afternoon peak traffic periods. To obtain the most accurate forecast and to be consistent with the requirements of the massDOT procedures, when available, the values in the fitted curves in the Trip Generation Manual were used to forecast trips to and from the proposed mixed residential and retail site for daily, and AM & PM peak hours. The ITE trip generation charts are presented in the Technical Appendix section of this report. The resulting trips and their directional distribution for this site are shown in the following Table 3.

ITE	E Trip Ge	neration f	or 36 Uni	ts of Mid-	-Rise Apa	artments (LU 231) 8	& 1,581 s	f of Retail (L				
36 Apartments ITE Land Use Code 231													
1	Daily	%In	%Out	AM Pk	%In	%Out	PM Pk	%In	%Out				
Rate-Trips/Unit	3.1*	50%	50%	0.2	39%	61%	0.28	44%	56%				
Trips	117*	58	59	7	3	4	10	4	6				
*Estimated based on massDOT K factor of 0.09 1,581 sf Apparel Store ITE Land Use Code 876													
[Daily	%In	%Out	AM Pk	%In	%Out	PM Pk	%In	%Out				
Rate-Trips/1,000 sf	12.4*	50%	50%	0.38	59%	41%	1.12	42%	58%				
Trips	20*	10	10	1	1	0	2	1	1				
*Estimated based on massDOT K factor of 0.09													
Grand Total Trips	137*	68	69	8	4	4	12	5	7				
	*Estimated h	ased on mass	DOT K factor	of 0.09									

TABLE 3 5)

As can be seen in the above Table 3, the total number of new trips expected to be generated by the proposed residential and retail development results in the highest traffic during afternoon peak period. In standard traffic engineering practice, the critical peak period trips are usually used to evaluate the worst-case scenario. Also, since the area intersections' traffic volumes are greatest during PM peak hour, the PM peak is the critical peak. Nonetheless, both peak traffic periods were evaluated for this development.

Trip Distribution and Assignment

Because such factors as population density, land use, availability of major highways in the area, and other demographics that make up the traffic patterns within a community, the directional distribution of the projected site-generated trips to and from the proposed development site was based on the existing traffic patterns within the immediate vicinity of the site and based on the knowledge of local traffic patterns. The turning movement traffic counts for the intersections within the study area are good indicators of the traffic patterns in this area.

Using this information, the projected new site-generated trips from the above Table 3 are assigned to each approach of these intersections. As shown in Table 3 above, during AM peak period, a sum of four vehicles will be arriving at the proposed site and four vehicles will be departing from the site in the northerly and southerly directions from both driveways along Meade and Grosvenor Streets. During PM peak period, a total of five vehicles are expected to arrive and seven vehicles will depart from the proposed site driveways. Finally, a total of 68 vehicles will be arriving and 69 vehicles will be leaving from the proposed site during a 24-hour period on an average weekday. The following Figure 5 shows the above-mentioned distribution of trips associated with the proposed site along the streets in the study area and the site driveways.



Figure 5 - Trip Generation and Distribution

Site Access, Circulation and Parking

Site access and internal traffic circulation were evaluated as part of assessing the proposed residential and retail development. Access to the proposed site will be achieved through a driveway from Meade Street that connects to the underground garage and a second driveway from Grosvenor Street to access the at-grade garage. The proposed driveways will provide full access to all parking spaces in the underground and at-grade parking spaces. The underground access driveway will have a pavement width of 24 feet while the at-grade driveway will have a pavement width of 28 feet, both of which are designed to accommodate two-way traffic.

The magnitude of parking spaces that will be needed for the proposed residential development was projected by using the latest (6th) edition of the *Parking Generation Manual*, also published by the ITE. A copy of the relative page is included in the Appendix section of this report.

Based on the ITE *Parking Generation Manual*, the rates at which mid-rise multifamily residential developments generate demand for parking vary depending upon the location of the project. The demand for off-street parking is greatest for facilities located in suburban areas primarily due to the lack of public transportation and long distances from daily conveniences. Based on the ITE *Parking Generation manual*, the 85th percentile or peak period parking demand rate for mid-rise multifamily residential developments in dense multi-use urban areas not close to rail transit is 1.29 parking spaces per dwelling unit. However, the average parking demand is 0.93 parking spaces per dwelling unit. As stated earlier, a total of 40 parking spaces are proposed for this site. Thus, the proposed parking supply is calculated at 1.11 spaces per unit. The proposed number of parking spaces is 12% greater than the national average rate of 0.93 and 12% lower than the 85th percentile (peak) demand on a weekday. Given the Transportation Demand Management program highlighted below, it is anticipated the actual demand for off-street parking will be even lower than that of the ITE *Parking Generation manual* during 85th percentile (peak) demand.

Therefore, based on the above assessment, it is concluded that sufficient parking spaces will be provided.

Transportation Demand Management

The proposed development at 39 Lamartine Street, which includes a total of 36 apartment units, will be benefiting from many City conveniences when it comes to different modes of transportation. The proposed site is located approximately a half mile from the Transportation Hub that provides access to all Worcester Regional Transit Authority (WRTA) bus routes, as well as Greyhound intercity buses. The Transportation Hub is also connected to Worcester Union Station at Washington Square that provides services via Amtrak and the MBTA to points east, including Boston. Thus, this TDM program is largely geared toward the use of these public transportation services and other modes such as pedestrian walkability, bicycling, etc. Therefore, it is suggested the consideration of advancement of the following improvements as a part of the Project, which are commensurate with the predicted impact of the Project on the transportation infrastructure and are focused on safety and encouraging the use of alternative modes of transportation instead of single-occupancy vehicles:

Public Transportation is a large component of this TDM program. There are WRTA bus stops located within 4-5 minutes of walking distance of the proposed site. Since this area is being developed with many new residential uses, this distance could be further reduced to less than a two-minute walk by providing additional bus stops on Green Island Boulevard. Therefore, WRTA should be consulted about establishing additional bus routes and stops in the general area of this and other future developments. These current bus stops that include WRTA Route 4 with 15-minute headways, two of which are located on Harding Street at Lafayette Street and Lamartine Street for inbound direction, and one is located on Millbury Street at Lafayette Street for outbound direction. Passengers can also take the inbound bus to the Transportation Hub and transfer to other routes in any direction. Alternatively, the residents of the proposed development could walk 10-13 minutes directly to the Transportation Hub or Union Station to transfer to other parts of the city or for commuting purposes particularly to points east, as far as the City of Boston by utilizing the MBTA train services. Therefore, the need for owning a motor vehicle and available off-street parking facilities is minimal. Also, to further discourage the use of personal vehicles, it is recommended that all information about the above-mentioned public transportation services such as maps of bus routes, bus and MBTA schedules, and fares should be made available to the future residents of the proposed site. Additionally, it is recommended that WRTA be consulted to provide a dynamic monitor, either inside or outside the building, that displays live information relative to available public transportation. Below is a bus route map that shows the existing bus stops in the vicinity of the proposed development as well as an MBTA map showing the stops along its route to Boston.

39 Lamartine Street Worcester, MA



MBTA Train Route



Walkability is considered a major part of Healthy Transportation mode and an advantage to the future residents of the proposed development, particularly since all streets to and from the proposed development site have sidewalks and proper handicap ramps. The proposed site is bound by Lamartine Street to the north, Meade Street to the east, Grosvenor to the west, and finally, the Worcester Department of Inspectional Services to the south. Lamartine Street leads to Harding and Millbury Streets in the easterly direction and to Green Island Boulevard and Hermon Street in the westerly direction. All the above-mentioned streets provide proper sidewalks and handicap ramps making travel on foot to many points of interest practicable. Therefore, no additional pedestrian accommodation is warranted, further minimizing the need for owning personal motor vehicles and the need for off-street parking facilities.

Bicycling is also considered a Healthy Transportation mode for the future residents of the proposed site, particularly since a number of the streets in close proximity of the proposed site feature dedicated bike lanes including Lamartine Street between Quinsigamond Avenue and Green Island Boulevard, Green Island Boulevard, Harding Street, and Millbury Street. Some other nearby streets that do not have dedicated bike lanes may also be candidates for being marked with Sharrow symbols to let motorists know they have to share the road with bicyclists. To further reduce the demand for motor vehicle ownership and on/off-street parking, the proposed site plan provides for a secure indoor bicycle parking facility. Additionally, it is recommended that an outdoor bicycle rack be provided to accommodate at least 6-8 bicycles.

Accessible Parking should be an important component of this TDM program as some residents will ultimately own personal motor vehicles that would need off-street parking accommodation. Although this demand will be significantly lower than in other locations with lower density and less accessibility to public transportation, they will require off-street parking. On-street parking should be made available for the retail portion of the proposed site as they are always used for short-term parking. Presently, there are no on-street parking restrictions on Meade Street, except a dedicated handicap parking space in front of number 12 Meade Street, a 100-foot distance on the west side of Meade Street from a point just north of the Worcester Inspectional Services Department entrance in the northerly direction, and 250 feet on the east side of the street from Lamartine Street southerly. Parking is not allowed on the south side of Lamartine Street. To accommodate the needs of the retail use component of this development, it is important to allow a 50-foot distance for shortterm parking as shown on the site plan. This will require a petition to the City Council for a change in the existing ordinances for this section of Meade Street. Additionally, to better manage the available on-site parking and to further discourage the need for personal car ownership, the proponent may have to assign each space only to the few residents who may own a vehicle, or even charge a premium. Finally, during special events and for the purpose of moving in/out of these apartments, if on-street parking should be needed, a special consideration should be requested from the City DTM's Parking Control section.

Ride-Sharing Mode is a valuable form of transportation that could further reduce the need for personal vehicle ownership, and thus, reduce traffic on area streets as well as decrease the need for on and off-street parking facilities. On-street parking is currently available on most of all three streets bordering the proposed site. Therefore, ride-sharing vehicles can use the nearby on-street parking to pick up/drop off passengers, as they are only short-term parking. Ride-share vehicles can also use the internal off-street parking spaces if on-street parking should not be available. Consequently, no special parking space designation would be necessary.

Work-at-Home accommodation should be included within the proposed project. Although the COVID-19 pandemic has ended, a considerable portion of the workforce continues to work from home as both employers and employees have become accustomed to the practice of the work-at-home concept. Therefore, the work-at-home accommodation may take the form of providing a meeting space and potentially a business office in the common (lobby/lounge) area of the building with such amenities as available internet, a computer and a printer.

Transportation Coordinator can be an asset for the new residents of the proposed development. Therefore, a transportation coordinator is recommended. The coordinator who may also have other responsibilities, can coordinate all components of the abovementioned TDM program such as providing the new residents with a welcome packet and information relative to public transportation, off-street parking arrangements, and bicycle storage.

In conclusion, given the location of the proposed site, the availability of a strong public transportation accommodation and the available walkable and cyclable streets, this project and other similar projects should have little or no impact on the area traffic.

4 TRAFFIC OPERATIONS

Measuring existing traffic volumes and projecting future traffic volumes quantify traffic flow within the study area. To assess the quality of traffic flow, intersection capacity analyses were performed to assess existing baseline conditions and for projected future design year (2029) conditions with and without development of the proposed residential project. Intersection capacity analyses provide an indication of how well roadway facilities and their components serve the traffic demands placed upon them. This section includes potential on-site and off-site mitigation improvements should any be deemed necessary to minimize the impact of the proposed residential development on the area streets.

Traffic operations measures

Level of Service (LOS) analysis was performed to determine the quality of traffic flow throughout the study area using criteria based on the Highway Capacity Manual and its computer software.

The LOS designation shows how well roadways, and their components process traffic placed upon them. Like a report card, LOSs are given letter designations from "A" to "F". LOS "A" represents the best operating conditions, while LOS "F" represents the worst. Typically, LOS "D" is considered acceptable during peak hour conditions, but LOS "E" may also be acceptable under some circumstances.

The LOS designation is reported differently for signalized and unsignalized intersections. For an unsignalized intersection, the Highway Capacity analysis assumes that through traffic on major roadways is not affected by traffic on side streets (streets with lower volumes and/or ones under stop sign control). Therefore, a LOS designation is typically calculated for the controlled movements (minor street approaches and major street left-turn movements). As described in the following paragraphs, capacity or LOS analyses were considered for year 2024 existing, year 2029 future no build, and year 2029 future build conditions for morning and evening peak hour periods at the above-mentioned intersections within the study area.

Existing Conditions

Intersection capacity analyses were performed for five intersections in the study area during morning and evening peak traffic periods. These intersections are considered the only locations in the vicinity of the proposed site that may be affected by the traffic expected to be generated by the proposed mixed-use development. All five intersections are unsignalized and are stop controlled.

The analysis concluded that LOS "A" is calculated for all controlled approaches of these intersections during both AM and PM peak periods and LOS "A" for the Intersection Capacity

Utilization during both peak periods. A summary of the intersection analyses results for existing conditions is shown below in Table 5.

Future Conditions

Capacity analyses for the future year peak hour traffic operations were performed for the year 2029 volumes during both peak periods with and without the proposed mixed-use development project in place. A summary of the intersection analyses results for both future no-build and future build conditions is also shown below in Table 5.

As noted earlier in this report, based on the *massDOT* Traffic Volume and Classification data, area streets are included in group U4-U7 for the Growth Factor and Seasonal Factor. Based on roadways in group U4-U7, the yearly growth rate for this group of roadways is calculated at 2% per year. Therefore, an adjustment factor of 2% per year was used, and the collected TMCs were further increased by 10% to reflect adjusted volumes for the future year 2029. Additionally, the Worcester Planning & Regulatory Division was consulted to identify potential nearby future developments that may have an impact on the intersections within the study area. Five projects were identified, and their locations were considered too far from the proposed development, thus it was determined that their traffic didn't reach any of the intersections within the study area. However, the traffic from a 12-unit apartment residential development at 10 Grosvenor Street by Polar Views, LLC was taken into consideration. The following Table 4 shows the trip generation and distribution for the proposed site at 10 Grosvenor Street. The projected traffic volumes from this development were also added to the year 2029 volumes to represent future No Build conditions. Figure 6 shows the volumes for the future no-build conditions for the intersections in the study area.

	Daily	%In	%Out	AM Pk	%In	%Out	PM Pk	%In	%Out
Rate-Trips/Unit	2.93	50%	50%	0.28	15%	85%	0.26	73%	27%
Trips	35	17	18	3	1	2	3	2	1

Table 4 – Trip Generation and Distribution for 10 Grosvenor Street 12 Apartments ITE Land Use Code 221

Build traffic volumes were determined by projecting site-generated traffic volumes and distributing those volumes over the intersections within the study area, and finally, adding them to the future no-build conditions volumes. The following Figure 7 shows future build conditions traffic volumes for these intersections.



Figure 6 - Turning Movement Counts, Future No Build Conditions

Figure 7 - Turning Movement Counts, Future Build Conditions



The intersection LOSs for the year 2029 no-build conditions were calculated for the controlled approaches of the intersections within the study area. It is expected the northbound approach of the intersection of Lamartine and Meade Streets will be operating at LOS "B" during evening peak period. All other controlled intersection approaches will continue to operate at LOS "A". Finally, the Intersection Capacity Utilization will also remain at LOS "A" for all intersections.

To assess the potential traffic impact of the proposed development on these intersections, all traffic from the proposed development site was assigned to the approaches of these five intersections. This should result in the assessment of these intersections under the worst-case scenario. The above Figures 2, 6 and 7 show the volumes at these intersections for both the morning and evening peak hours under existing, future no-build, and future build conditions.

The intersection analyses for the year 2029 build conditions were performed for all five intersections. The analysis revealed that under future build conditions, all controlled approaches of these intersections will continue to operate at the same level as those under future no-build conditions and the same Intersection Capacity Utilization LOS "A" as under no-build conditions.

Again, the above-mentioned LOS "B" for the northbound approach of Meade Street at its intersection with Lamartine Street under future no-build and future build conditions is indicative of little or no delays of traffic at these intersections and represents little or no impact associated with the development of the proposed development project. A summary of intersection analyses for all five intersections is provided below in Table 5. The computer printouts of the above-mentioned analysis are included in the Technical Appendix of this report.

T	able 5
Level Of Service Analy	sis Results Summary

Intersection	AM Pe	Existin	Existing Future No Build Future Build					Build		Intersection	AM Pea	Existin	g	Future	No Buil	d	Future Build		
Lamartine/Meade Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB	Lafayette/Meade Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB
Vplume/Capacity	0.05	0.01	0.05	0.06	0.01	0.06	0.06	0.01	0.06	Vplume/Capacity	0	0	0.01	0.01	0	0.01	0.01	0	0.01
Approach Delay	0	0.5	9.5	0	0.5	9.6	0	0.7	9.6	Approach Delay	1	0	8.9	1	0	8.9	1.2	0	8.9
LOS			A			A			A	LOS			A			A			A
Int Capacity Utilization LOS	A			A			Α			Int Capacity Utilization LOS	A			A			Α		·
	PM Pea	k									PM Peak								
	EB	WB	NB	EB	WB	NB	EB	WB	NB		EB	WB	NB	EB	WB	NB	EB	WB	NB
Vplume/Capacity	0.08	0.01	0.03	0.09	0.01	0.04	0.09	0.01	0.04	Vplume/Capacity	0	0	0.02	0	0	0.02	0	0	0.02
Approach Delay	0	0.8	10	0	0.8	10.3	0	0.8	10.4	Approach Delay	0.3	0	9	0.3	0	9.1	0.4	0	9.1
LOS			Α			В			В	LOS			Α			Α			A
Int Capacity Utilization LOS	A			А			A			Int Capacity Utilization LOS	Α			Α			Α		
	~																		
	AM Pea	k									AM Peal	k 🛛							
Lamartine/Grosvenor Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB	Lafayette/Grosvenor Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB
Vplume/Capacity	0.04	0.01	0.02	0.05	0.01	0.02	0.05	0.01	0.03	Vplume/Capacity	0.01	0	0.01	0.01	0	0.01	0.01	0	0.01
Approach Delay	0	0.7	9.3	0	0.7	9.5	0	0.7	9.5	Approach Delay	1.4	0	8.9	1.4	0	9	1.5	0	9
LOS	2		Α			Α	2		A	LOS			A			A			A
Int Capacity Utilization LOS	Α			A			A			Int Capacity Utilization LOS	A			Α			A		
	P <mark>M P</mark> ea	k									PM Peal	k							
Lamartine/Grosvenor Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB	Lamartine/Grosvenor Sts	EB	WB	NB	EB	WB	NB	EB	WB	NB
Vplume/Capacity	0.08	0	0.02	0.09	0	0.03	0.09	0	0.03	Vplume/Capacity	0.01	0	0.01	.01	0	0.01	0.01	0	0.01
Approach Delay	0	0.2	10	0	0.2	10.2	0	0.2	10.2	Approach Delay	1.3	0	9.1	1.4	0	9.2	1.5	0	9.2
LOS			A			В			В	LOS			Α			A			A
Int Capacity Utilization LOS	A A A						A			Int Capacity Utilization LOS	A			Α			A		

	AM Pea	Existin	g		Future	No Bui	ld		Future	Build		
Lamartine/Hermon/Green Island	EB	WB	NB	SB	EB	WB	NB	SB	EB	WB	NB	SB
Vplume/Capacity	0.21	0.15	0.18	0.07	0.24	0.17	0.2	0.09	0.24	0.17	0.2	0.09
Approach Delay	8.4	8.1	8.1	7.9	8.7	8.3	8.4	8.1	8.7	8.3	8.4	8.1
LOS	A	Α	Α	А	A	Α	А	Α	А	A	Α	A
Int Capacity Utilization LOS	A				A				А			
	PM Pea	k										
Lamartine/Hermon/Green Island	EB	WB	NB	SB	EB	WB	NB	SB	EB	WB	NB	SB
Vplume/Capacity	0.14	0.13	0.2	0.23	0.16	0.14	0.23	0.26	0.16	0.15	0.23	0.26
Approach Delay	8.2	8.1	8.2	8.7	8.5	8.2	8.5	9	8.5	8.3	8.5	9
LOS	A	Α	A	А	A	A	Α	Α	Α	A	A	٨
					A				A			

39 Lamartine Street Worcester, MA

5 FINDINGS

This traffic study has been conducted to evaluate the potential traffic impacts associated with the proposed mixed-use residential and commercial development site located on the south side of Lamartine Street in the city of Worcester, Massachusetts. This study includes the evaluation of five intersections which are in close proximity of the site and are likely to be impacted by the proposed development project. Evaluation of these intersections, which were identified by consultation with Worcester DTM, and which is intended to identify capacity constraints was performed for existing, future no-build, and future build conditions. Future analyses have determined that the site-generated traffic volumes are not significant, and they can easily be accommodated with the existing roadways, their intersections, and the site driveways off Meade Street and Grosvenor Street. These analyses demonstrated that with the additional traffic volumes associated with the proposed development, the Intersection Capacity Utilization LOSs will stay the same as those of the existing and the future no build conditions LOS "A". The analysis concluded that the approaches of all five intersections are and will continue to operate at LOS "A" except the northbound approach of Meade Street at Lamartine Street which will be operating at LOS "B" under future no build and build conditions during afternoon peak period.

Conclusion & Recommendations

It is concluded that of the five intersections evaluated, the intersection of Lafayette Street and Meade Street which has had two accidents over a five-year period has experienced an accident rate greater than average for unsignalized intersections, and therefore, a remedy is warranted at this intersection. It is believed that the on-street parking on the north side of Lafayette Street at Meade Street may have contributed to these two accidents. The Intersection of Lafayette Street, Lodi Street, Hermon Street and Green Island Boulevard had an accident rate of 0.35 which is significantly lower than average for unsignalized intersections. The remaining three intersections had no accidents reported during this five-year period.

The available sight distances at the proposed driveways will allow motorists to safely enter and exit the flow of traffic on both Meade Street and Grosvenor Street.

The volumes of traffic associated with the proposed development are not considered significant, and therefore, the surrounding streets have ample capacity to safely serve the anticipated additional traffic. The level of service evaluation presented above is an indicator of the quality of traffic flow through the area. This evaluation indicates that no impact is expected from the proposed development and the LOS will not fall below "A" for all approaches of the intersections studied except for the northbound approach of Meade Street at Lamartine Street which will be operating at LOS "B" during PM peak period. Also, since there is no opportunity for landscaping along the property lines on either Lamartine Street, Meade Street or Grosvenor Street, the site distances are not expected to be impacted. To maintain optimum safety and efficiency, the following recommendations, most of which are identified in the Transportation

Demand Management program in this report, should be considered.

- It is recommended that the city of Worcester reinstall the missing stop signs mentioned in this report.
- It is recommended that on-street parking on the north side of Lafayette Street be prohibited from the Meade Street intersection westerly for 50 feet.
- Information regarding public transportation services should be made available to residents and include maps, schedules and fare information.
- A "welcome packet" should be provided to new residents providing the name and contact information for the transportation coordinator and detailing available public transportation services, bicycle and walking alternatives, and other commuting options.
- The proposed secure bicycle parking consisting of both weather-protected bicycle parking and exterior bicycle racks should be always maintained.
- Consult with the MWRTA to discuss options to establish transit service to the streets near the project site such as Green Island Boulevard and Lamartine Street.
- Work-at-home accommodation should be included within the Project and may take the form of meeting space and a business office in the common or lobby area.
- Assign a transportation coordinator or superintendent of the building for the proposed project who may also have other responsibilities to coordinate the TDM program.

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Technical Appendix



Peak Hour	Analysis	From	07:00	AM to	08:45	AM -	Peak 1	1 of	1
Peak Hour	for Each	Appro	bach B	egins a	at:				

1 Cult 1 loui	101 -		pprou		gino a																				X
_	08:00 AM					07:45 AM					07:45 AM					07:00 AM					07:45 AM	r i			
+0 mins.	2	10	0	3	15	1	0	17	2	20	1	12	13	4	30	0	0	0	0	0	8	19	12	0	39
+15 mins.	1	5	0	1	7	3	0	<mark>13</mark>	3	19	0	10	12	1	23	0	0	0	0	0	3	20	6	0	29
+30 mins.	3	8	3	2	16	0	4	14	3	21	1	19	11	4	35	0	0	0	0	0	9	21	9	3	42
+45 mins.	3	8	1	2	14	3	1	14	0	18	4	14	9	4	31	0	0	0	0	0	7	20	9	0	36
Total Volume	9	31	4	8	52	7	5	58	8	78	6	55	45	13	119	0	0	0	0	0	27	80	36	3	146
% App. Total	17.3	59.6	7.7	15.4		9	6.4	74.4	10.3		5	46.2	37.8	10.9		0	0	0	0		18.5	54.8	24.7	2.1	
PHF	.750	.775	.333	.667	.813	.583	.313	.853	.667	.929	.375	.724	.865	.813	.850	.000	.000	.000	.000	.000	.750	.952	.750	.250	.869
Cars	9	30	4	8	51	7	5	54	7	73	6	55	44	13	118	0	0	0	0	0	27	78	36	3	144
% Cars	10 0	96. 8	10 0	10	98.1	10 0	10 0	93. 1	87. 5	93.6	10 0	10 0	97. 8	10 0	99.2	0	0	0	0	0	10 0	97. 5	10 0	10 0	98.6
Trucks	0	1	0	0	1	0	0	4	1	5	0	0	1	0	1	0	0	0	0	0	0	2	0	0	2
% Trucks	0	3.2	0	0	1.9	0	0	6.9	12. 5	6.4	0	0	2.2	0	0.8	0	0	0	0	0	0	2.5	0	0	1.4



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

1 Out 1 loui		acii A	pproc		gins u																				
	04:15 PM					04:00 PM				111	04:15 PM					04:15 PM					04:00 PM				
+0 mins.	5	13	6	6	30	0	2	15	2	19	4	8	16	4	32	0	0	0	0	0	1	14	9	2	26
+15 mins.	2	16	9	3	30	3	4	7	1	<mark>15</mark>	3	16	9	3	31	1	0	0	0	1	1	15	7	1	24
+30 mins.	1	16	9	5	31	4	4	8	2	18	3	8	13	5	29	0	0	0	0	0	2	15	5	1	23
+45 mins.	8	20	14	3	45	1	0	7	0	8	4	17	16	4	41	0	1	0	0	1	3	10	6	0	19
Total Volume	16	65	38	17	136	8	10	37	5	60	14	49	54	16	133	1	1	0	0	2	7	54	27	4	92
% App. Total	11.8	47.8	27.9	12.5		13.3	16.7	61.7	8.3	11425993	10.5	36.8	40.6	12		50	50	0	0		7.6	58.7	29.3	4.3	10.00503
PHF	.500	.813	.679	.708	.756	.500	.625	.617	.625	.789	.875	.721	.844	.800	.811	.250	.250	.000	.000	.500	.583	.900	.750	.500	.885
Cars	16	65	38	17	136	8	10	37	5	60	14	49	54	16	133	1	1	0	0	2	7	52	26	4	89
% Cars	10 0	10 0	10 0	10 0	100	10 0	10 0	10 0	10 0	100	10 0	10 0	10 0	10 0	100	10 0	10 0	0	0	100	10 0	96. 3	96. 3	10 0	96.7
Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	1	0	3
% Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3.7	3.7	0	3.3



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	07:45 AM			08:00 AM			07:45 AM		_
+0 mins.	0	30	30	1	1	2	18	0	18
+15 mins.	1	21	22	2	1	3	19	0	19
+30 mins.	3	33	36	3	0	3	14	0	14
+45 mins.	1	29	30	3	2	5	19	0	19
Total Volume	5	113	118	9	4	13	70	0	70
% App. Total	4.2	95.8		69.2	30.8		100	0	
PHF	.417	.856	.819	.750	.500	.650	.921	.000	.921
Cars	5	112	117	8	4	12	66	0	66
% Cars	100	99.1	99.2	88.9	100	92.3	94.3	0	94.3
Trucks	0	1	1	1	0	1	4	0	4
% Trucks	0	0.9	0.8	11.1	0	7.7	5.7	0	5.7



Cars

% Cars

Trucks

% Trucks

100



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

5.	07:45 AM			08:00 AM			07:45 AM		
+0 mins.	0	27	27	2	2	4	19	1	20
+15 mins.	1	21	22	7	1	8	20	1	21
+30 mins.	0	29	29	4	5	9	14	1	15
+45 mins.	2	26	28	2	8	10	17	2	19
Total Volume	3	103	106	15	16	31	70	5	75
% App. Total	2.8	97.2	0	48.4	51.6		93.3	6.7	
PHF	.375	.888	.914	.536	.500	.775	.875	.625	.893
Cars	3	102	105	15	15	30	69	5	74
% Cars	100	99	99.1	100	93.8	96.8	98.6	100	98.7
Trucks	0	1	1	0	1	1	1	0	1
% Trucks	0	1	0.9	0	6.2	3.2	1.4	0	1.3



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:00 PM			05:00 PM			04:15 PM		
+0 mins.	3	32	35	4	1	5	23	1	24
+15 mins.	4	31	35	0	0	0	25	1	26
+30 mins.	2	28	30	1	1	2	20	2	22
+45 mins.	3	29	32	1	3	4	33	1	34
Total Volume	12	120	132	6	5	11	101	5	106
% App. Total	9.1	90.9		54.5	45.5		95.3	4.7	
PHF	.750	.938	.943	.375	.417	.550	.765	.625	.779
Cars	12	120	132	6	5	11	101	5	106
% Cars	100	100	100	100	100	100	100	100	100
Trucks	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0


Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	07:00 AM			07:00 AM			08:00 AM		
+0 mins.	1	0	1	0	0	0	3	9	12
+15 mins.	0	0	0	0	0	0	3	10	13
+30 mins.	3	0	3	0	0	0	2	5	7
+45 mins.	0	0	0	0	0	0	1	11	12
Total Volume	4	0	4	0	0	0	9	35	44
% App. Total	100	0		0	0		20.5	79.5	
PHF	.333	.000	.333	.000	.000	.000	.750	.795	.846
Cars	4	0	4	0	0	0	8	35	43
% Cars	100	0	100	0	0	0	88.9	100	97.7
Trucks	0	0	0	0	0	0	1	0	1
% Trucks	0	0	0	0	0	0	11.1	0	2.3



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	04:00 PM			04:00 PM			04:00 PM		
+0 mins.	2	0	2	0	0	0	1	13	14
+15 mins.	0	0	0	0	0	0	1	15	16
+30 mins.	4	0	4	0	0	0	0	15	15
+45 mins.	2	0	2	0	0	0	1	21	22
Total Volume	8	0	8	0	0	0	3	64	67
% App. Total	100	0		0	0		4.5	95.5	
PHF	.500	.000	.500	.000	.000	.000	.750	.762	.761
Cars	8	0	8	0	0	0	3	64	67
% Cars	100	0	100	0	0	0	100	100	100
Trucks	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

	08:00 AM			07:00 AM			08:00 AM		
+0 mins.	1	0	1	0	0	0	2	10	12
+15 mins.	2	0	2	0	0	0	3	13	16
+30 mins.	1	0	1	0	0	0	3	5	8
+45 mins.	1	0	1	0	0	0	1	11	12
Total Volume	5	0	5	0	0	0	9	39	48
% App. Total	100	0		0	0		18.8	81.2	
PHF	.625	.000	.625	.000	.000	.000	.750	.750	.750
Cars	5	0	5	0	0	0	8	38	46
% Cars	100	0	100	0	0	0	88.9	97.4	95.8
Trucks	0	0	0	0	0	0	1	1	2
% Trucks	0	0	0	0	0	0	11.1	2.6	4.2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak nour for Each App	ouch Degino ut								
	05:00 PM			04:00 PM			04:00 PM		
+0 mins.	1	0	1	0	0	0	3	14	17
+15 mins.	1	0	1	0	0	0	0	16	16
+30 mins.	1	0	1	0	0	0	4	12	16
+45 mins.	3	0	3	0	0	0	1	19	20
Total Volume	6	0	6	0	0	0	8	61	69
% App. Total	100	0		0	0		11.6	88.4	
PHF	.500	.000	.500	.000	.000	.000	.500	.803	.863
Cars	6	0	6	0	0	0	8	61	69
% Cars	100	0	100	0	0	0	100	100	100
Trucks	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0

le: 20260001	Average	NB,	7 7	10	3 4	4	10	24	1 33	1 44	3 64	1 52	31	1 46	1 48	09 00	0 62	1 58	1 67	1 62	1 38	30	24	37	3 16	14	2 845	107	8:00	64	4:00	19 01	107
Site Coo	Week	SB,	14	w	w	U	1	20	30	44	48	54	52	99	76	80	90	114	130	124	74	65	46	51	28	20	1262	2	11:00	65	4:00	130	2
		NB,	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	-1 3					
	Sunday	SB,	*	•	*	•	٠	•	•	*	•	•	•	٠	•	•	•	•	•	*	٠	•	•	ŧ	×	•	0	0					0
		ЧВ,	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	8					
	Saturday	SB, N		*	•	*	•	*	×	¥	×	¥	•	×	¥	×	¥	•	•	¥		×	•	*	×	*	0	0					0
		В,	*	*	*	*	*	¥	*	*		*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	60 B					6
	Friday	SB, N	×	*	*	*	*	×	*	*		*	*	×	*		×	*	ĸ	×	•	×	*	¥)	×	*	0	0					0
		В,	9	6	2	5	11	22	34	44	69	48	33	44	51	70	64	69	74	99	39	33	37	44	18	19	891	15 8	8:00	59	4:00	74	e.
	Thursday	SB, N	14	9	6	8	9	19	36	40	44	47	69	66	75	82	16	109	147	123	78	73	48	46	32	23	1297	2188	10:00	69	4:00	147	2188
	٨	B,	8	10	5	3	10	26	32	45	70	57	29	49	45	50	59	57	60	58	37	27	12	30	15	8	802		8:00	70	4:00	60	
	Wednesda	SB. N	14	11	00	3	8	20	41	48	53	60	34	64	77	62	84	120	114	125	70	57	45	56	23	18	1232	2034	11:00	64	5:00	125	2034
		8,	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	8 0	6	10			
	Tuesday	SB, N	*	٠	×	•		*	•	•	•		•	×	•	•	•	٠	•	•	*	•	•	•	•	•	0	0					0
			*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	6	_	4			
n Street f Green Street ster, MA	Monday	SB, NB	*	*	*	*	*	*	*	*	•	*	*	٠	*	*	•	*	•	*	*	*	*	*	•	*	0	0					0
Location Hermol Location North o City/State: Worces	7/15/2024	Time	12:00 AM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	9:00	10:00	11:00	12:00 PM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	00:6	10:00	11:00	Total	Day	AM Peak	Volume	PM Peak	Volume	Comb Total

39 Lamartine Street Worcester, MA

Site Code: 20260001

Accurate Counts 978-664-2565

Location : Hermon Street Location : North of Green Street City/State: Worcester, MA Direction: Combined

7/18/2024	0 0			- 0 40	> 12 -	> 15 -	> 18 -	> 21 -	> 24 -	> 27 -	> 30 -	> 33 -	> 36 -	> 00	
Time	MPH	> 3 - 6 MPH	>6-9 MPH	>9-12 MPH	MPH	> 39 MPH	Total								
12:00 AM	0	0	1	1	1	6	1	6	3	1	0	0	0	0	20
1:00	0	0	0	3	2	4	2	2	1	1	0	0	0	0	15
2:00	0	0	0	0	3	4	2	2	0	0	0	0	0	0	11
3:00	0	0	0	1	6	0	2	3	0	0	1	0	0	0	13
4:00	0	0	0	0	2	5	4	3	2	1	0	0	0	0	17
5:00	0	0	0	0	1	9	15	11	3	1	1	0	0	0	41
6:00	0	0	4	2	5	15	11	16	12	3	1	1	0	0	70
7:00	0	0	7	3	2	4	18	22	20	8	0	0	0	0	84
8:00	0	0	7	2	6	10	24	21	20	11	2	0	0	0	103
9:00	0	0	5	3	3	9	19	35	11	5	3	2	0	0	95
10:00	0	0	2	5	7	15	27	28	13	1	1	3	0	0	102
11:00	0	0	1	7	12	14	35	18	13	5	3	2	0	0	110
12:00 PM	0	0	0	3	8	18	34	29	23	8	1	2	0	0	126
1:00	0	0	3	1	4	17	40	37	27	16	5	1	0	1	152
2:00	0	0	4	11	12	18	44	36	24	9	3	0	0	0	161
3:00	0	0	3	7	14	16	40	45	25	14	4	0	0	0	168
4:00	0	0	5	8	16	14	43	60	53	15	5	1	1	0	221
5:00	0	0	4	6	12	20	27	53	38	18	6	2	2	1	189
6:00	0	0	0	0	2	12	26	36	25	8	5	2	1	0	117
7:00	0	0	1	0	3	4	22	21	34	16	4	0	1	0	106
8:00	0	0	0	0	5	2	14	31	19	9	3	0	2	0	85
9:00	0	0	2	2	1	7	24	29	17	6	1	0	1	0	90
10:00	0	0	0	0	5	8	15	8	9	3	2	0	0	0	50
11:00	0	0	1	0	4	5	13	7	6	5	1	0	0	0	42
Total	0	0	50	65	136	236	502	559	398	164	52	16	8	2	2188
		P	ercentile	15th	50th	85th	95th								
			Speed	15	20	25	28								
	Mean	Speed (A	(verage)	21.7											
	10	MPH Pace	e Speed	18-27											
		Number	in Pace	1484											
	100	Percent	in Pace	71.0%											
	N	umber >	21 MPH	1199											
	P	ercent >	21 MPH	54.8%											
Grand Total	0	0	120	118	279	515	990	988	729	328	110	29	10	6	4222
Stats		Pe	ercentile	150	SUT	85th	95th								
	1226		Speed	15	20	25	28								
	Mean	Speed (A	(verage)	21.5											
	101	10 MPH Pace Speer		18-27											
		Number	in Pace	2866											
		Percent	an Pace	09.0%											
	N	umper >	21 MPH	2200											
	Р	Percent > 21 MPH		52.1%											

0260002	age	EB,	16	12	6	8	19	42	76	111	130	96	81	88	102	104	96	87	105	106	62	68	56	64	37	21	1613		8:00	130	5:00	106		
Site Code: 2	Week Aver	WB,	22	20	10	8	16	34	54	82	109	82	96	16	110	118	114	112	114	100	82	88	65	60	38	29	1660	3273	8:00	109	1:00	118	3273	
		3,	*	•	×	*	*	*	*	*	*	*	*	×	*	*	*	*	•	*	*	*	*	×	*	*	0	200 - E			ż	-		
	Sunday	WB, EE	*	×	×	×	•	×	*	*	*	*	*	×	*	•	*	*	•1	×	*	*	*	*	×	•	0	0					0	
		В,	*	۲	*	*	*	*	*	*	*	*	*	*	*	*	*	*	e	*		*	*	*	*	*	0				4			
	Saturday	WB, E	•	•	٠	*	•	٠	٠	٠	•	•	•	٠	•	•	•	٠	•	•	•	•	·	٠	٠	•	0	0					0	
			*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	8 9			-		2	
	day	EB,																										0					_	
	Fri	WB,	•	ŧ)	*			*	*	ð	*	•	•	*	ð.	*	*	*	t)	ð.	*	*	•	*	*		0							
			12	15	9	7	18	40	78	95	128	92	77	93	98	118	89	83	124	131	74	72	65	75	47	18	655		8:00	128	5:00	131		
	Irsday	EB			10	0	1.000	0		~	-	6	0	~		10	6	_	10		6	~	•					359	0	~	0	2	359	
	Thu	WB,	16	1	^a	1	2	30	5	80	104	79	6	108	12'	125	100	12	125	16	79	6	66	67	36	36	1704	ŝ	11:00	108	1:00	125	3	
		3,	19	8	12	10	20	43	75	127	132	101	85	82	106	06	102	91	86	81	84	64	46	53	27	24	1568	6 5	8:00	132	Md 0	106		
	Inesday	E	4	0	5	5	2	7	9	0	4	5	0	9	6	0	80	4	3	2	9	3	1	4	6	2	5	3183	0	4	0 12:00	8	3183	
	Wed	WB,	2	2	-		+	3	5	80	11	8	10	8	6	11	11	10	10	10	8	8	9	5	3	2	161	0)	8:0	11	2:0	11	0,	
		В,	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0							3,271
	Jesday	Ш		*	*	*	*		*	*	*	*		*	*	*	*	÷	*	*	*	*		*	*	*	0	0					0	AADT:
	Ţ	WB,																																
		В,	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	•	*	*	*	*	*	*	*	0	5			S.			3,271
Ireet Street AA	onday	ш	•	•	*		*	•	*	*	*		•		*	*			*	*	*			*		*	0	0					0	ADT:
t of Lod	Z	WB,																																
ocation Lam ocation Wes	7/15/2024	Time	12:00 AM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	00:6	10:00	11:00	12:00 PM	1:00	2:00	3:00	4.00	5:00	6:00	7:00	8:00	00.6	10:00	11:00	Total	Day	AM Peak	Volume	PM Peak	Volume	Comb Total	ADT

39 Lamartine Street Worcester, MA

Site Code: 20260002

Location : Lamartine Street Location : West of Lodi Street City/State: Worcester, MA Direction: Combined

noodon. oom	Diriod													
7/18/2024	0 - 15	> 15 -	> 20 -	> 25 -	> 30 -	> 35 -	> 40 -	> 45 -	> 50 -	> 55 -	> 60 -	> 65 -	> 70	2-65 0000
Time	MPH	20 MPH	25 MPH	30 MPH	35 MPH	40 MPH	45 MPH	50 MPH	55 MPH	60 MPH	65 MPH	70 MPH	MPH	Total
12:00 AM	0	1	3	9	15	1	2	0	0	0	0	0	0	31
1:00	0	0	4	12	12	4	2	0	0	0	0	0	0	34
2:00	0	0	1	6	4	1	0	0	0	0	0	0	0	12
3:00	0	0	2	10	4	3	0	0	0	0	0	0	0	19
4:00	0	0	2	17	12	6	2	0	0	0	0	0	0	39
5:00	2	3	4	19	29	9	3	1	0	0	0	0	0	70
6:00	2	6	10	39	49	18	4	1	0	0	0	0	0	129
7:00	19	22	59	56	20	2	0	0	0	0	0	0	0	178
8:00	11	29	33	86	56	15	2	0	0	0	0	0	0	232
9:00	5	9	31	53	55	17	1	0	0	0	0	0	0	171
10:00	10	9	36	68	38	8	0	0	0	0	0	0	0	169
11:00	3	15	52	79	36	14	2	0	0	0	0	0	0	201
12:00 PM	4	14	36	81	61	20	3	0	0	0	0	0	0	219
1:00	16	26	64	81	51	5	0	0	0	0	0	0	0	243
2:00	22	33	69	49	20	5	0	0	0	0	0	0	0	198
3:00	0	4	23	48	89	32	8	0	0	0	0	0	0	204
4:00	6	5	33	56	94	42	7	6	0	0	0	0	0	249
5:00	1	9	18	59	90	40	10	1	0	0	0	0	0	228
6:00	0	2	10	38	57	36	8	2	0	0	0	0	0	153
7:00	0	0	10	55	68	28	2	2	0	0	0	0	0	165
8:00	4	13	13	39	46	16	3	0	0	0	0	0	0	134
9:00	0	6	14	57	45	16	2	2	0	0	0	0	0	142
10:00	1	0	10	35	28	6	4	1	0	0	0	0	0	85
11:00	0	5	7	17	18	6	0	1	0	0	0	0	0	54
Total	106	211	544	1069	997	350	65	17	0	0	0	0	0	3359
		F	Percentile	15th	50th	85th	95th							
			Speed	21	28	34	37							
	Mea	n Speed ((Average)	28.9										
	10	MPH Pa	ce Speed	26-35										
		Numbe	er in Pace	1968										
		Percer	nt in Pace	61.0%										
		Number >	> 30 MPH	1429										
		Percent >	> 30 MPH	42.5%										
Grand Total	163	331	959	2062	2086	760	153	26	2	0	0	0	0	6542
Stats		F	Percentile	15th	50th	85th	95th							
			Speed	23	29	34	37							
	Mea	n Speed ((Average)	29.5										
	10	MPH Pa	ce Speed	26-35										
		Numbe	er in Pace	4113										
		Percer	nt in Pace	63.0%										
		Number > 30 MPH		3027										
		Percent >	> 30 MPH	46.3%										

erage	VB,	32	25	13	12	18	36	81	126	173	148	152	146	158	174	170	152	181	163	150	154	106	112	75	40	2597	~	8:00	173	4:00	181		
Week Av	Ľ,	12	0	00	00	00	20	58	63	20	66	17	66	72	84	19	108	109	102	70	64	47	54	28	19	1301	3898	10:00	77	4:00	109	3696	
QIV	WB,	*	*	*	*	*	*	*	*	*	*	*	4	*	*	*	*	*	*	*	*	*	*	*	*	0							
Sunday	Ē	*	*	*	*	*	*	*	*	*	*	*	*	*	•	*	*	*	*	*	*	*	*	*	*	0	0					0	
g	'n,	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	•	•	*	*	*	•	*	*	0						0	
Saturday	EG, V	*	*	*	*	*	•	*	×	*	٠	*	*	×	•	+	•	•	•	*	*	*	*	٠	*	0	0					0	
	ζ,	*	*	*	*	+	*	*	٠	*	*	*	•	*	•	*	*	*	*	*	*	*	*	*	*	0						i.	
Friday ED ME	EG, WE	÷	*	*	*		*	*	×	×	×	÷	×	*	•	*	*	×	*	×	×	×	×	×	÷	0	0					0	
(E	VUB,	25	26	2	18	21	40	81	114	150	160	140	156	179	161	154	156	187	167	147	173	138	141	69	51	2659		00:6	160	4:00	187		
Thursd	ĽĊ,	œ	0	9	00	0	18	59	55	73	69	82	99	11	96	82	104	123	102	76	99	48	62	34	19	1345	4004	10:00	82	4:00	123	4004	
ay	NG,	88	24	21	2	16	31	81	137	196	137	165	137	138	188	187	149	175	169	152	136	73	83	81	29	2540		8:00	196	1:00	188		
Wednesd	ЦΩ,	17	6	10	ດ	00	22	56	71	68	62	72	65	72	73	76	113	95	102	65	63	46	47	21	19	1261	3801	10:00	72	3:00	113	3801	
ę	'n	*	*	*	*	*	*	*	×	*	*	*	4	*	•	*	*	*	*	*	*	*	*	*	*	0							3,902
Tuesday	EБ, V	*	*	*	*	*	٠	*	×	*	٠	*	×	*	•	*	*	*	*	*	*	÷	÷	×	*	0	0					0	AADT:
	, ,	*	*	*	*	*	*	*	×	*	*	*	+	*	*	*	*	*	*	*	*	*	*	*	*	0							902
Monday	С. М.	*	*	*	*	*	*	*	*	*	*	*	*	*	•	*	*	*	*	*	*	*	*	*	*	0	0					0	ADT: 3,
5/2024 Time	1 Ime	00 AM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	9:00	10:00	11:00	M 00:	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	00:6	10:00	11:00	Total	Day	A Peak	/olume	1 Peak	olume'	b Total	ADT

39 Lamartine Street Worcester, MA

Location	: Lamartine Street
Location	: East of Lodi Street
City/State	e: Worcester, MA
Direction	Combined

7/18/2024	50 SER	7825 SR	5.82 8	1000	> 12 -	> 15 -	<mark>> 1</mark> 8 -	> 21 -	> 24 -	> 27 -	> 30 -	> 33 -	> 36 -	2012	
	0 - 3	> 3 - 6	> 6 - 9	> 9 - 12	15	18	21	24	27	30	33	36	39	> 39	
Time	МРН	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	мрн	Total
12:00 AM	0	0	0	0	3	8	14	5	2	0	1	0	0	0	33
1:00	0	0	1	1	6	6	9	1	2	2	1	0	0	1	35
2:00	0	0		1	4	0	1	2	2	1	0	0	0	1	26
1:00	0	0	0	0	2	4	11	0	2	4	0	0	0	0	20
4.00	0	0	0	2	2	4 5	21	10	10	7	1	0	0	0	50
6:00	0	0	0	2	5	15	36	30	30	12	3	1	0	0	140
7:00	0	0	0	3	22	27	49	30	24	11	3	0	0	0	169
8:00	0	0	3	10	18	25	78	38	33	14	4	0	0	0	223
9.00	0	0	4	4	26	39	68	47	21	16	4	0	0	0	229
10:00	Ō	0	1	12	32	55	65	23	16	11	6	0	0	1	222
11:00	0	0	0	9	22	51	65	42	22	7	4	0	0	0	222
12:00 PM	0	0	3	14	31	57	76	33	22	7	6	0	1	0	250
1:00	0	0	7	4	22	45	80	38	35	18	7	1	0	0	257
2:00	0	0	7	16	22	39	69	52	19	9	3	0	0	0	236
3:00	0	0	3	2	24	33	81	55	46	13	2	1	0	0	260
4:00	0	0	1	6	26	53	81	70	39	24	9	0	0	1	310
5:00	0	0	3	7	19	32	82	54	41	23	5	1	1	1	269
6:00	0	0	4	4	21	41	67	32	28	21	3	0	1	1	223
7:00	0	0	1	3	17	36	81	56	30	10	2	2	1	0	239
8:00	0	0	1	3	22	39	63	27	18	8	3	2	0	0	186
9:00	0	0	0	3	32	30	68	36	19	12	2	1	0	0	203
10:00	0	0	3	1	12	21	20	25	13	5	1	1	1	0	103
11:00	0	0	2	2	9	19	14	10	7	4	3	0	0	0	70
Total	0	0	45	115	399	684	1207	739	486	236	73	10	5	5	4004
		P	ercentile	15th	50th	85th	95th								
	Maan	Chood (A	Speed	20.6	19	24	28								
	10 N		Spood	16.25											
	10 1	Number	in Dace	2692											
		Percent	in Pace	70.0%											
	N	umber >	18 MPH	2761											
	P	ercent >	18 MPH	69.0%											
Grand Total	0	0	100	224	798	1312	2303	1484	976	433	133	23	7	12	7805
Stats		P	ercentile	15th	50th	85th	95th								
			Speed	15	19	24	27								
	Mean	Speed (A	verage)	20.6											
	10 N	10 MPH Pace Speed		16-25											
		Number	in Pace	5408											
		Percent	in Pace	70.0%											
	N	umber >	18 MPH	5371											
	P	ercent >	18 MPH	68.8%											

20260004	WR	16	16	00	7	14	40	49	50	84	62	74	88	76	80	76	75	88	69	69	52	49	40	28	22	1250		11:00	88	1:00	88		
Site Code: 2	Week Ave	15	໑	9	9	11	34	58	76	102	70	72	75	73	76	76	52	76	80	62	49	44	48	34	17	1221	2471	8:00	102	5:00	80	2471	
	day WR	*	*	×	*	*	*	*	*	*	×	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0				-		-	
	Sun	*	*	×	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	¥	*	*	0							
	aturday WR	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0					0	
	о Щ	*	*	*	*	*	*	*	*	*	*	*	*	*	×	*	*	*	*	*	*	*	*	*	*	0				č			
	Friday WR	*	4	*	*	*	*	*	*	*	*	*	*	*	×	*	*	*	*	*	*	*	*	-12	•	0	0					0	
	ä	17	13	4	7	100	31	50	48	78	57	65	00	83	113	83	74	98	69	74	49	47	40	36	28	282		00:	00	00:	13		
	Thursday FR M/R	14	10	4	9	6	37	54	66	97	67	80	85	77	66	79	52	85	105	63	53	51	52	43	13	1268 12	2550	8:00 11	97	5:00	105 1	2550	
	ay MR	16	18	13	7	10	48	48	51	06	68	82	76	69	83	69	76	78	69	64	55	51	41	19	17	1218		8:00	90	1:00	83		
	Wednesd	16	00	б	9	13	31	63	86	107	73	63	65	69	85	72	53	67	55	61	45	37	44	26	21	1175	2393	8:00	107	1:00	85	2393	
	V V/B	*	٠	¥	*	*	×	*	*	*	*	٠	۲	٠	×	*	*	*	*	*	*	٠	×	*	*	0						01 V 1	: 2,412
	Tuesda	*	×	*	*	*	*	*	*	*	×	9K	×	*	×	*	*	*	*	*	*	*	*	*	(a .)	0	0					0	AAUI
	AVB	*	*	*	*	*	*	*	*	*	*	*	*	*	¥	*	*	*	*	*	*	*	*	*	*	0						0170	2,412
Street f Lodi Street sster, MA	Monday	*	×	*	*	*	*	*	*	*	¥	*	*	*	ĸ	*	*	*	*	*	*	*	*	*	*	0	0					0	AUL
Location : Green Location : East o City/State: Worce	7/15/2024 Time	12:00 AM	1:00	2:00	3:00	4.00	5:00	6:00	2:00	8:00	9:00	10:00	11:00	12:00 PM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	00.6	10:00	11:00	Total	Day	AM Peak	Volume	PM Peak	Volume	Comb Total	AUI

39 Lamartine Street Worcester, MA

Location	: Green Street
Location	: East of Lodi Street
City/State	e: Worcester, MA
Direction	Combined

7/18/2024	1	1212		2.00	> 12 -	> 15 -	> 18 -	> 21 -	> 24 -	> 27 -	> 30 -	> 33 -	> 36 -		
	0-3	> 3 - 6	> 6 - 9	> 9 - 12	15	18	21	24	27	30	33	36	39	> 39	
12:00 AN		MPH	MPH	MPH	MPH 1	MPH	MPH		MPH	MPH	MPH	MPH	MPH	МРН	l otal
12.00 AN		0	0	2	0	4	0	11	4	2	1	0	0	0	23
2:00		0	0	2	0	2	4 3	1	2	0	0	0	0	0	23
3:00		0	1	2	2	2	2	2	2	0	0	0	0	0	13
4.00	0	0	0	0	1	1	12	8	4	0	1	0	0	0	27
5:00	0 0	0	0	Ō	4	5	17	17	15	9	1	0	0	õ	68
6:00	0 0	0	0	0	3	10	28	34	17	7	4	0	0	1	104
7:00	0 0	0	0	1	8	7	29	33	29	4	3	0	0	0	114
8:00	0 0	0	4	8	8	11	43	45	36	12	5	0	0	3	175
9:00	0 0	0	2	7	6	13	29	28	23	12	3	0	0	1	124
10:00	0 0	0	4	2	14	12	39	44	24	5	0	0	0	1	145
11:00	0 0	0	6	4	16	24	51	51	20	9	0	1	1	2	185
12:00 PN	1 0	0	0	4	13	20	41	44	25	9	2	1	0	1	160
1:00	0 0	0	5	7	15	34	49	36	26	6	1	0	0	0	179
2:00	0 0	0	1	3	6	25	47	41	29	8	1	1	0	0	162
3:00	0 0	0	2	3	11	11	30	34	27	5	2	1	0	0	126
4:00	0 0	0	0	1	14	33	48	38	35	11	3	0	0	0	183
5:00	0 0	0	0	1	12	23	38	40	38	15	4	1	1	1	174
6:00	0 0	0	1	0	9	8	40	28	30	17	3	1	0	0	137
7:00	0 0	0	2	0	5	12	19	28	25	9	2	0	0	0	102
8:00	0	0	0	0	6	10	24	25	21	8	4	0	0	0	98
9:00	0	0	0	U	4	8	32	33	10	5	0	0	0	0	92
10:00		0	0	1	8	11	26	19	8	0	0	0	0	0	19
Tota		0	29	10	169	201	675	9	159	162	41	6	2	10	2550
1018	0	P	ercentile	15th	50th	85th	95th	050	400	102	41	0	2	10	2000
			Speed	16	20	24	28								
	Mean	Speed (A	Average)	21.7											
	10	MPH Pac	e Speed	18-27											
		Number	in Pace	1811											
		Percent	in Pace	75.0%											
	N	lumber >	21 MPH	1338											
	F	Percent >	21 MPH	52.5%											
Grand Tota	0	0	39	81	311	562	1349	1287	859	338	79	18	3	17	4943
State	5	P	ercentile	15th	50th	85th	95th								
		_	Speed	16	20	24	28								
	Mean	Speed (A	Average)	21.8											
	101	Numatra	e Speed	18-27											
		Derecat	in Pace	75 00/0											
	N	lumber >		2601											
		Percent >	21 MPH	52.6%											
		or oon the	- 1 100 11	02.070											

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(ccu	976

Location : Lodi Street Location : South of Lamartine Street City/State: Worcester, MA

	ge	Mon - Sun	0	2	2	2	-	9	10	16	14	24	19	19	30	16	18	14	20	20	12	ω	12	9	e	4	278		00:6	24	12:00 PM	30
	Avera	Mon - Fri	0	0	2	2	-	9	10	16	14	24	19	19	30	16	18	14	20	20	12	œ	12	9	0 N	4	278		00:6	24	12:00 PM	30
	7/21/2024	Sun	*	*	*	*	*	æ	*	*	*	*	*	*	*	*	×	*	*	*	*	*	*	*	*	×	0	0.0%				
	7/20/2024	Sat	*	*	*	*	*	æ	*	*	*	*	*	*	*	*	×	*	*	*	*	*	*	*	*	×	0	0.0%				
	7/19/2024	Fri	*	*	*	*	*	4	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	4	0	0.0%				
	7/18/2024	Thu	0	~	2	~	F	4	12	14	<mark>თ</mark>	21	16	19	33	16	23	13	33	23	о О	11	10	თ	4	5	289	51.7%	00 :6	21	12:00 PM	33
	7/17/2024	Wed	-	4	2	2	-	7	00	17	20	28	22	19	27	16	14	16	00	18	15	5	13	en en	5	2	270	48.3%	9:00	28	12:00 PM	27
	7/16/24	Tue	*	*	*	đ	*	*	*	*	*	đ	*	*	*	*		₫.	*	*	*	+	*	* ∶	*	*	0	0.0%				
ester, MA	7/15/24	Mon	*	*	*	*	*	×	*	*	*	*	*	*	*	*	×	*	*	*	*	*	*	*	*	*	0	0.0%				
City/State: Worc	7/15/2024	Time	12:00 AM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	6:00	10:00	11:00	12:00 PM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	6:00	10:00	11:00	Total	Percent	AM Peak	Volume	PM Peak	Volume

Location	: Lodi Street
Location	: South of Lamartine Street
City/State	: Worcester, MA
Direction	SB,

	7/18/2024		~ ~	• •	- -	> 12 -	> 15 -	> 18 -	> 21 -	> 24 -	> 27 -	> 30 -	> 33 -	> 36 -		
	Time	0 - 3 MPH	> 3 - 6 MPH	> 6 - 9 MPH	> 9 - 12 MPH	15 MPH	18 MPH	21 MPH	24 MPH	27 MPH	30 MPH	33 MPH	36 MPH	39 MPH	> 39 MPH	Total
	12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	1:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
	2:00	0	0	0	0	2	0	0	0	0	0	0	0	0	0	2
	3:00	0	0	0	0	0	1	0	. 0	0	0	0	0	0	0	1
	4:00	0	0	0	0	0	0	1	0	0	0	0	0	0	0	1
	5:00	0	0	1	0	2	0	1	0	0	0	0	0	0	0	4
	6:00	0	0	3	3	0	4	0	2	0	0	0	0	0	0	12
	7:00	0	0	4	4	1	2	1	1	1	0	0	0	0	0	14
	8:00	0	0	3	2	1	2	1	0	0	0	0	0	0	0	9
	9:00	0	0	2	4	6	6	2	0	1	0	0	0	0	0	21
	10:00	0	0	2	4	4	3	3	0	0	0	0	0	0	0	16
	11:00	0	0	2	5	8	1	3	0	0	0	0	0	0	0	19
	12:00 PM	0	0	5	5	12	6	2	1	0	0	1	0	0	1	33
	1:00	0	0	3	3	4	3	2	0	1	0	0	0	0	0	16
	2:00	0	0	2	5	8	3	2	1	0	0	1	0	0	1	23
	3:00	0	0	2	0	6	2	2	1	0	0	0	0	0	0	13
	4:00	0	0	6	6	5	7	1	2	1	0	0	0	2	3	33
	5:00	0	0	2	5	10	2	3	0	1	0	0	0	0	0	23
	6:00	0	0	1	2	0	1	4	1	0	0	0	0	0	0	9
	7:00	0	0	3	2	3	0	0	0	0	0	0	0	2	1	11
	8:00	0	0	2	2	2	1	1	0	0	0	0	0	0	2	10
	9:00	0	0	0	0	3	4	0	0	0	0	0	0	0	2	9
	10:00	0	0	1	0	0	1	2	0	0	0	0	0	0	0	4
3	Total	0	0	15	52	2	10	32	10	5	0	2	0	0	10	200
-	TOTAL	0	P	ercentile	15th	50th	85th	95th	10	5	0	2	U	4	10	209
				Speed	8	13	16	20								
		Mean	Speed (A	Average)	15.5		10									
		10 1	MPH Pac	e Speed	8-17											
			Number	in Pace	194											
			Percent	t in Pace	69.0%											
		N	umber >	12 MPH	192											
8		F	ercent >	12 MPH	66.4%											22
(Grand Total	0	0	90	123	131	99	61	17	7	3	4	0	5	19	559
8	Stats		P	ercentile	15th	50th	85th	95th								
				Speed	8	12	16	19								
		Mean	Speed (A	Average)	15.2											
		101	MPH Pac	e Speed	10-19											
			Number	in Pace	382											
			Percent	t in Pace	69.0%											
		N	umber >	12 MPH	346											
		F	ercent >	12 MPH	61.9%											

						Accurate C 978-664-2	2565	ts						
Location : Gros Location : Lam City/State: Word	svenor Street Betweel artine Street and Lafa cester, MA	an ayette Street											Site Code:	20260006
7/15/2024	Monday	Tuesday		Wednesday		Thursday		Friday		Saturday		Sunday	Week Ave	erage
Time	SB, NB,	SB, NB,		SB, NE	°,	SB, NB,		SB, NB,	1998 1	SB, NB,		SB, NB,	SB,	NB,
12:00 AM	*	*	9	0	3	0	0	*	*	÷	*	•	0	2
1:00	*	*	*	0	0	0	0	*	*	*	*	*	0	0
2:00	*	*	*	0	-	-	-	*	*	×	*	*	0	F
3:00	*	*	*	0	-	۲-	-	*	*	*	*	*	0	-
4:00	*	*	3	F	.	0	+	*	+		4	*	0	1
5:00	*	*	*	0	0	0	c	*	*	*	*	*	0	2
6:00	*	*	*	en en	ო	ი	2	*	*	*	*	*	с С	4
7:00	*	*	*	2	თ	1	00	*	*	*	*	*	0	00
8:00	4 4	*	×	5	11	2	9	×	*	×	æ	*	4	80
9:00	*	*	*	4	10	0	12	*	*	*	*	*	ю	11
10:00	*	*	*	2	7	5	2	*	*	*	*	*	4	4
11:00	*	*	*	7	0	4	13	*	*	*	*	*	9	11
12:00 PM	*	*	*	-	9	ro O	00	ĸ	*	×	*	*	2	7
1:00	*	*	*	5	7	IJ	12	*	*	*	*	*	ъ U	10
2:00	*	*	*	11	0	4	14	*	*	×	*	*	00	12
3:00	*	*	*	0	9	4	00	*	*	×	*	*	0	6
4:00	*	*	*	-	00	e	15	*	*	*	*	*	0	12
5:00	*	*	,	60	11	Ð	13	•	*	*	*	•	9	12
6:00	*	*	*	11	14	9	11	*	*	*	*	*	œ	12
7:00	*	*	*	9	7	12	20	*	*	*	*	*	თ	14
8:00	*	*	*	1	4	4	10	*	*	*	*	*	2	7
9:00	*	*	9	4	00	ო	Q	*	4	*	4	*	4	9
10:00	*	*	*	2	<mark>л</mark>	2	9	*	*	×	*	*	2	9
11:00	*	*	*	Ļ	ო	0	4	*	*	*	*	*	0	4
Total	0	0	0	75	147	70 1	178	0	0	0	0	0	72	164
Day	0	0	3	222		248		0		0	8	0	236	
AM Peak				11:00	8:00	10:00 11.	00:						11:00	00:6
Volume				1	11	Q	13				-		9	11
PM Peak Volume				2:00	6:00 14	7:00 7. 12	20						6 6	7:00
Comb Total	0	0		222	1	248		0		0		0	236	
ADT	ADT: 235	AADT: 23	35											

Location : Grosvenor Street Between
Location : Lamartine Street and Lafayette Street
City/State: Worcester, MA
Direction: Combined

7/18/2024	1997 4390	1944	040 800		> 12 -	> 15 -	> 18 -	> 21 -	> 24 -	> 27 -	> 30 -	> 33 -	> 36 -	55559	
	0-3	> 3 - 6	> 6 - 9	> 9 - 12	15	18	21	24	27	30	33	36	39	> 39	
10:00 AM	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	MPH	lotal
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1.00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2.00	0	0	0	0	0	0	2	0	0	0	0	0	0	0	2
4:00	0	0	2	0	1	0	0	0	0	0	0	0	0	0	4
5:00	0	0	0	0	0	0	1	2	0	0	0	0	0	0	3
6:00	0	0	0	1	1	1	2	2	0	1	0	0	0	0	0
7:00	0	0	0	2	2	3	2	2	0	0	0	0	0	0	q
8:00	0	0	0	-	2	1	3	2	1	1	0	0	0	0	8
9.00	0	0	1	4	5	4	0	0	0	0	0	0	0	0	14
10:00	0	0	0	2	2	1	2	0	0	0	0	0	0	0	7
11:00	0	0	4	1	3	1	4	2	2	0	0	0	0	0	17
12.00 PM	0	0	0	2	1	3	3	1	1	0	0	0	0	0	11
1.00	0	0	3	4	2	2	3	2	0	1	0	0	0	0	17
2:00	0	0	0	4	4	3	4	3	0	0	0	0	0	0	18
3:00	0	0	1	2	3	4	1	1	0	0	0	0	0	0	12
4:00	0	0	1	3	5	5	4	0	0	0	0	0	0	0	18
5:00	0	0	1	2	2	5	5	2	1	0	0	0	0	0	18
6:00	0	0	2	1	3	4	4	1	2	0	0	0	0	0	17
7:00	0	0	14	5	3	3	4	1	2	0	0	0	0	0	32
8:00	0	0	0	4	4	1	4	1	0	0	0	0	0	0	14
9:00	0	0	3	0	1	3	0	1	0	0	0	0	0	0	8
10:00	0	0	1	3	2	2	0	0	0	0	0	0	0	0	8
11:00	0	0	0	1	0	2	0	0	1	0	0	0	0	0	4
Total	0	0	33	41	44	48	46	23	10	3	0	0	0	0	248
		P	ercentile	15th	50th	85th	95th								
			Speed	9	15	20	23								
	Mean	Speed (A	Average)	15.5											
	10 N	MPH Pac	e Speed	13-22											
		Number	in Pace	149											
		Percent	in Pace	63.0%											
	N	umber >	15 MPH	130											
	P	ercent >	15 MPH	52.4%			0.5	10	17						170
Grand I otal	0	0	57	11	84	94	95	43	17	3	0	0	0	0	470
Stats		P	ercentile	15th	50th	85th	95th								
	Maan	Canad ()	Speed	15.0	15	20	25								
	10 Miean		verage)	12.0											
	101	Number	in Pace	305											
		Percent	in Pace	65.0%											
	N	umber >	15 MPH	252											
	P	ercent >	15 MPH	53.6%											

Location : Mea Location : Lam City/State: Won	de Street Between artine Street and Lafa cester, MA	ayette Street											Site Code: 202	60007
7/15/2024 Time	Monday SB NB	Tuesday SR NB	Wedne	sday	Thursday SB NB		Friday SR NB		Saturday SB NB		Sunday SR NB		Week Averag	e a
12:00 AM	*	*	* 22,	0	0	0	*	*	*	+	*	*	1	0
1:00	*	*	*	0	0	0	*	*	•	*	*	*	0	0
2:00	*	*	•	0	0	0	*	*	•	*	*	*	0	0
3:00	*	*	*	0	0	0	*	*	*	*	*	*	-	0
4:00	*	*	•	0	~	0	*	*	*	*	*	*	0	0
5:00	*	*	•	0		0	*	*	*	*	*	*	0	0
6:00	*	*	*	0	2	~	*	*	*	*	*	*	2	0
7:00	*	*	9	с О	0	3	*	*	*	*	*	*	80	3
8:00	*	*	ر م *	2	9	3	*	*	*	*	*	*	4	2
00:6	*	*	*	7	10	-	*	*	*	*	*	*	8	-
10:00	*	*	* 14	0	10	0	*	*	*	*	*	*	12	0
11:00	*	*	*	2	18	~	*	*	*	*	*	*	13	-
12:00 PM	*	*	*	n	9	~	*	*	*	*	*	*	8	2
1:00	*	*	5	0	10	0	*	*	*	*	*	*	80	~
2:00	*	*	*	0	11	-	*	*	*	*	*	*	10	0
3:00	*	*	о *	0	15	~	*	*	*	*	*	*	12	0
4:00	*	*	*	0	4	-	*	*	*	*	*	*	4	0
5:00	*	•	* °	0	S	0	ŧ	4	ł	\$	*	4	<mark>ю</mark>	0
6:00	*	*	*	0	9	0	*	*	*	*	*	*	4	0
7:00	*	*	*	0	9	0	*	*	*	*	*	*	Q	0
8:00	*	*	*	0	7	0	*	*	*	*	*	*	4	0
00:6	*	*	*	0	ი	0	*	*	*	*	*	*	2	0
10:00	*	*	۳	0	~	0	*	*	*	*	*	*	2	0
11:00	*	*	۰ ۶	0	4	0	*	*	*	*	*	*	4	0
Total	0	0	100	12	133 1	e	0	0	0	0	0	0	115	10
Day	0	0	11:	2	146	8	0		0	8	0	2	125	
AM Peak Volume			10:00 14	2:00 3	11:00 7:0 18	0 0							11:00 13	7:00 3
PM Peak			12:00 PM	12:00 PM	3:00 12:00 PI			8				100	3:00 12:0	0 PM
Volume			11	3	15			_					12	2
Comb Total	0	0	11	2	146		O		0		0		125	
AU	AUL: 128	AAUI: 12	D											

39 Lamartine Street Worcester, MA

Site Code: 20260007

Location	: Meade Street Between
Location	: Lamartine Street and Lafayette Street

City/State: Worcester, MA Direction: Combined

0-3 > 3-6 > 6-9 > 9-12 15 18 21 24 27 30 33 36 39 >	39
Time MPH	PH Total
12:00 AM 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0
1:00 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0
2:00 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0
3:00 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0
4:00 0 0 0 0 0 0 1 0 0 0 0	0 1
5:00 0 0 0 0 0 1 0 0 0 0 0	0 1
6:00 0 0 1 1 0 1 0 0 0 0 0 0 0	0 3
7:00 0 0 8 2 1 0 0 0 1 0 0 0 0	0 12
8:00 0 0 7 0 1 1 0 0 0 0 0 0 0	0 9
9:00 0 0 7 1 1 1 1 0 0 0 0 0 0	0 11
10:00 0 0 1 1 4 3 1 0 0 0 0 0 0	0 10
11:00 0 0 6 3 7 2 1 0 0 0 0 0 0	0 19
12:00 PM 0 0 2 0 1 0 3 1 0 0 0 0 0	0 7
1:00 0 0 3 1 1 1 4 0 0 0 0 0 0	0 10
2:00 0 0 1 4 4 1 2 0 0 0 0 0 0	0 12
3:00 0 0 7 2 3 2 2 0 0 0 0 0 0	0 16
4:00 0 0 1 1 0 2 1 0 0 0 0 0 0	0 5
5:00 0 0 0 2 0 1 0 0 0 0 0	0 3
6:00 0 0 0 0 1 3 0 1 0 1 0 0 0	0 6
7:00 0 0 0 0 0 2 3 1 0 0 0 0 0	0 6
8:00 0 0 0 0 3 1 2 0 1 0 0 0 0	0 7
9:00 0 0 1 0 1 1 0 0 0 0 0 0 0	0 3
10:00 0 0 0 0 0 0 1 0 0 0 0 0	0 1
<u>11:00 0 0 0 0 1 1 1 1 0 0 0 0 0</u>	0 4
Total 0 0 45 16 31 22 24 5 2 1 0 0 0	0 146
Percentile 15th 50th 85th 95th	
Speed 0 12 18 20	
Mean Speed (Average) 13.5	
10 MPH Pace Speed 7-16	
Number in Pace 101	
Percent in Pace 72.0%	
Number > 12 MPH 85	
Percent > 12 WPH 56.2%	0 250
Giand Total 0 0 72 37 36 34 44 9 3 1 0 0 0	0 256
Stats Felcendie 15th 55th 55th 55th	
Mean Speed (Average) 13.5	
10 MPH Pare Sneed 7.16	
Number in Pace 185	
Percent in Pace 73.0%	
Number > 12 MPH 149	
Percent > 12 MPH 57.8%	

Location : Lafayette Street Between Location : Meade Street and Grosvenor Street City/State: Worcester, MA

		n - Sun	10	00	9	4	9	26	33	31	40	42	58	44	50	56	50	54	09	54	55	49	41	36	30	18	861		10:00	58	4:00	60
	Average	Mon - Fri Mo	10	8	9	4	9	26	33	31	40	42	58	44	50	56	50	54	60	54	55	49	41	36	30	18	861		10:00	58	4:00	60
	7/21/2024	Sun	*	*	*	*	×	*	*	*	*	*	*	*	*	-*	*	*	*	*	*	-*	*	*	*	*	0	0.0%				
	7/20/2024	Sat	*	*	*	*	4	*	*	*	*	*	*	*	*	*	*	*	*	×	*	*	*	*	*	*	0	0.0%				
	7/19/2024	Fri	*	*	*	*	4	*	*	*	*	*	*	*	*	*	*	*	*	×	*	*	*	*	*	*	0	0.0%				
	7/18/2024	Thu	13	10	7	ъ С	Q	24	28	32	37	43	54	43	61	51	54	49	55	56	59 59	<mark>63</mark>	50	42	32	25	889	51.7%	10:00	54	12:00 PM	61
	7/17/2024	Wed	7	9	4	3	5	27	38	30	44	40	61	45	38	61	47	60	65	51	51	45	32	31	28	12	831	48.3%	10:00	61	4:00	65
	7/16/24	Tue	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	¥	*	*	*	*	¥	*	0	0.0%				
ster, MA	7/15/24	Mon	¥	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0.0%				
City/State: Worce	7/15/2024	Time	12:00 AM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	9:00	10:00	11:00	12:00 PM	1:00	2:00	3:00	4:00	5:00	6:00	7:00	8:00	9:00	10:00	11:00	Total	Percent	AM Peak	Volume	PM Peak	Volume

Location : Lafayette Street Between Location : Meade Street and Grosvenor Street City/State: Worcester, MA Direction: EB,

7/18/2024					> 12 -	> 15 -	> 18 -	> 21 -	> 24 -	> 27 -	> 30 -	> 33 -	> 36 -		
.	0-3	> 3 - 6	> 6 - 9	> 9 - 12	15	18	21	24	27	30	33	36	39	> 39	Tetel
12:00 AM	MPH	IVIPH 0	MPH					MPH			MPH 3		IVIPH	MPH	1001
12.00 AM	0	0	2	1	0	0	0	1	4	3	0	0	0	0	10
2:00	0	0	0	0	1	1	3	1	0	0	1	0	0	0	7
3:00	0	0	0	0	0	1	0	0	3	1	0	0	0	0	5
4.00	0	0	0	1	2	0	0	1	1	. 1	0	0	0	0	6
5:00	0	0	1	1	2	0	5	9	2	3	1	õ	0	0	24
6:00	0	0	3	1	2	1	6	5	5	4	1	0	0	0	28
7:00	0	0	1	5	3	1	5	8	5	2	1	1	0	0	32
8:00	0	0	2	1	3	5	5	6	8	4	2	1	0	0	37
9:00	0	0	2	1	4	3	10	13	5	3	2	0	0	0	43
10:00	0	0	2	0	4	11	20	12	2	1	2	0	0	0	54
11:00	0	0	2	2	5	6	17	3	7	1	0	0	0	0	43
12:00 PM	0	0	6	6	5	9	19	10	3	3	0	0	0	0	61
1:00	0	0	3	1	4	3	18	13	5	2	0	1	1	0	51
2:00	0	0	1	1	3	5	13	14	11	5	0	1	0	0	54
3:00	0	0	4	1	7	8	12	11	4	1	1	0	0	0	49
4:00	0	0	3	1	5	7	14	15	6	2	2	0	0	0	55
5:00	0	0	4	5	10	3	10	12	8	2	0	1	1	0	56
6:00	0	0	3	5	6	9	16	7	7	2	2	1	0	1	59
7:00	0	0	0	1	5	5	16	10	11	4	1	0	0	0	53
8:00	0	0	2	4	6	7	9	13	7	1	1	0	0	0	50
9:00	0	0	4	0	4	3	15	8	2	1	3	0	1	1	42
10:00	0	0	4	2	3	2	7	6	5	0	1	2	0	0	32
<u>11:00</u>	0	0	1	0	2	1	6	7	2	3	2	0	0	1	25
Total	0	0	52	42	87	92	227	185	114	50	26	8	3	3	889
		P	ercentile	15th	50th	85th	95th								
		Constant ()	Speed	20 4	6	12	25								
	iviean		Average)	17.26											
	101		in Deed	F20											
		Dorcont	in Pace	63 0%											
	N	umbor		709											
	P	ercent >	15 MPH	79.6%											
Grand Total	0	0	122	124	229	220	397	314	188	75	35	9	4	3	1720
Stats		P	ercentile	15th	50th	85th	95th								
			Speed	8	13	20	24								
	Mean	Speed (A	Average)	19.2											
	10 1	MPH Pac	e Speed	16-25											
		Number	in Pace	994											
		Percent	t in Pace	59.0%											
	N	umber >	15 MPH	1245											
	F	ercent >	15 MPH	72.4%											



INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN :	Worcester			COUNT DA	TE :	Jul-24
DISTRICT :	UNSIGN	ALIZED :	Х	SIGNA	LIZED :	
		~ 111	TERSECTION			
		114	TEROLETION			
MAJOR STREET :	Lafayette Str	eet				
MINOR STREET(S) :	Meade Stree	t				
	7					
	3					
	1 1			Meade St		
INTERSECTION	North					
DIAGRAM	NOTUT		8			
(Label Approaches)						
			Ļ			
		Lafayette St				-
		67	►			
			PEAK HOUF			Total Dook
APPROACH :	1	2	3	4	5	Hourly
DIRECTION :	EB	WB	SB			Approach Volume
PEAK HOURLY VOLUMES (AM/PM) :	67	0	8			75
"K " FACTOR :	0.090		ECTION ADT APPROACH	(V)= TOTA I VOLUME:	AL DAILY	833
TOTAL # OF CRASHES :	2	# OF	5	AVERA CRASHES	GE # OF PER YEAR (0.40
		TEARS:		Α):	
CRASH RATE CALCU	JLATION :	1.32	RATE =	<u>(A*1,0</u> (V	000,000) * 365)	
Comments : Much high	ner than avera	ge of 0.61 for	unsignalized	intersections	in Dist 3 of m	assDOT
Project Title & Date:	39 Lamartine	Street reside	ential developr	nent August	2024	



INTERSECTION CRASH RATE WORKSHEET

CITY/TOWN :	Worcester			COUNT DA	ГЕ:	Jul-24
DISTRICT :	UNSIGN	ALIZED :	X	SIGNA	LIZED :	
		~ IN	TERSECTION	I DATA ~		
MAJOR STREET :	Green Island	l Bo <mark>ulevard</mark>				
MINOR STREET(S) :	Lamartine St	reet				
	16					
	↑	Hermon St				
INTERSECTION	North	136	60	Green Island	Blvd	
DIAGRAM	North					
(Label Approaches)	Lamartine St	4		- 133	Lamartine St	1
		90	11			
			Lodi St			
			PEAK HOUF	VOLUMES		
		2	3	4	5	Total Peak
	NE	2	3 N	4	5	Total Peak Hourly Approach
DIRECTION : PEAK HOURLY	NE	2 W	3 N	4 SW	5 S	Total Peak Hourly Approach Volume
DIRECTION : PEAK HOURLY VOLUMES (AM/PM) :	NE 90	2 W 133	3 N 0	4 SW 60	5 S 136	Total Peak Hourly Approach Volume 419
DIRECTION : PEAK HOURLY VOLUMES (AM/PM) : " K " FACTOR :	NE 90 0.090	2 W 133 INTERSI	3 N 0 ECTION ADT APPROACH	4 SW 60 (V) = TOTA VOLUME :	5 S 136 NL DAILY	Total Peak Hourly Approach Volume 419 4,656
DIRECTION : PEAK HOURLY VOLUMES (AM/PM) : " K " FACTOR : TOTAL # OF CRASHES :	NE 90 0.090 3	2 W 133 INTERSI # OF YEARS :	3 N 0 ECTION ADT APPROACH	4 SW 60 (V) = TOTA VOLUME : AVERA CRASHES A	5 S 136 AL DAILY GE # OF PER YEAR ():	Total Peak Hourly Approach Volume 419 4,656 0.60
DIRECTION : PEAK HOURLY VOLUMES (AM/PM) : "K" FACTOR : TOTAL # OF CRASHES : CRASH RATE CALCU	NE 90 0.090 3	2 W 133 INTERSI # OF YEARS : 0.35	3 N 0 ECTION ADT APPROACH 5 RATE =	4 SW 60 (V) = TOTA VOLUME : AVERA CRASHES A (A*1.0 (V)	5 S 136 NL DAILY GE # OF PER YEAR (): ^{100,000}) 365)	Total Peak Hourly Approach Volume 419 4,656 0.60

Project Title & Date: 39 Lamartine Street Residential Development August 2024

					2				, Mass	MAS sachusetts Dep	SDOT partment of Transportation	Most	Vehicle	Road	Ambient	Weather	At	Distance	Distance	Distance	Distance	Vulnerabi	×	Y
Number			Ŋ		Seventy Reported	venicies	s injunes	injuries		Prior to Crash		Harmful Events	Configura tion	Surface Condition	Light	Condition	Roadway Intersecti on	From Nearest Roadway Intersecti on	From Nearest Milemark er	From Nearest Exit	From Nearest Landmark	e User Type	Cooordin ate	Cooordin ate
468994	48 WORCEST 3 ER 2	81-Jan- 2019	12:08 PM	Not Reported	Not Applicabl e		2	0	0 Angle	V1: Travellin, straight ahead / V2: Turning left	V1: Not Reported / V2: Not g Reported	V1:(Collis on with motor vehicle in traffic) / V2:(Collis on with motor vehicle in traffic)	i V1:(Light truck(van, mini-van, pickup, sport ii utility)) / V2:(Pass enger car)	Ice	Daylight	Clear/Cle ar	LAMARTI NE STREET LODI STREET	LAMARTI NE / STREET / LODI STREET	81				174995.8 1224996	889371.8 1258123
48995	65 WORCEST 1 ER 2	11-Feb- 2020	6:20 AM	Property damage only (none injured)	No Apparent Injury (O)		1	0	0 Single vehicle crash	V1: Travellin straight ahead	V1: N g	V1:(Collis on with fence)	i V1:(Pass enger car)	Wet	Daylight	Rain	LAMARTI NE STREET LODI STREET	LAMARTI NE / STREET / LODI STREET					174995.8 122	889371.8 126
52532	97 WORCEST (ER 2	06-Mar- 2023	2:31 PM	Property damage only (none injured)	No Apparent Injury (O)		2	0	0 Angle	V1: Travellin straight ahead / V2: Travellin straight ahead	y1: /V2: g	V1:(Collis on with motor vehicle in traffic) / V2:(Collis on with motor vehicle in traffic)	ii V1:(Pass enger car) / V2:(Pass enger ii car)	Dry	Dark - lighted roadway	Clear	HERMON STREET Rte / LAMARTI NE STREET Rte	HERMON STREET Rte / LAMARTI NE STREET Rte					174995.8 122	889371.8 126
490578	6 WORCEST 2 ER 2	7-Oct-	1:10 PM	Unknown	Not reported		4	0	0 Angle	V1: Turning left /V2: Travelling straight ahead / V3: Parked / V4: Parked	V1: Not Reported / V2: Not Reported / V3: Not Reported / V4: Not Reported	V1:(Collisi on with motor vehicle in traffic) / V2:(Collisi on with motor vehicle in traffic) / V3:(Collisi on with motor vehicle traffic) / V4:(Collisi on with motor vehicle traffic) / V4:(Collisi	V1:(Pass [enger car) / V2:(Light truck(van, mini-van, pickup, sport vditky)) / V3:(Pass enger car) / V4:(Light in truck(van mini-van, isi pickup, sport utility)) in	Dry (Daylight	Clear/Cle ar	MEADE STREET / LAFAYET TE STREET	MEADE STREET / LAFAYET TE STREET					175144.3 8 752 8	89191.2
512336	4 WORCEST 0 ER 2	7-May- 022	3:45 PM	Property damage only (none injured)	No Apparent Injury (O)		3	0	0 Angle	V1: Travellir straight ahead / V2: Travellir straight ahead / V3: Parked	V1: Not Reported / V2: Not ng Reported / V3: Not Reported	V1:(Coll on with motor vehicle traffic) / V2:(Coll on with motor vehicle traffic) / V3:(Coll on with motor vehicle traffic)	isi V1:(Pass enger car) / in V2:(Pass enger isi car) / V3:(Pass enger in car) isi	s Dry	Daylight	t Clear	LAFAYI TE STREE MEADE STREE	ET LAFAYE TE T / STREE MEADE T STREE	ET T				175144 752	3 889191.2 501

Massachusetts Highway Department Statewide Traffic Data Collection 2019 Weekday Seasonal Factors

Factor Group	JAN	FEB	MAR	APR	MAY	NUL	JUL	AUG	SEP	OCT	NON	DEC	Axle Factor
R1	1.22	1.14	1.12	1.06	1.00	0.96	0.87	0.85	0.96	0.99	1.04	1.12	0.85
R2	0.95	0.96	0.98	0.97	0.97	0.93	0.97	0.94	0.96	0.90	0.92	0.93	0.96
R3	1.15	1.06	1.07	1.00	0.89	0.88	0.89	0.89	0.95	0.92	1.02	1.01	0.97
R4-R7	1.09	1.09	1.11	1.02	0.96	0.92	0.89	0.89	0.99	0.98	1.09	1.13	0.98
U1-Boston	1.03	1.01	0.98	0.94	0.94	0.92	0.95	0.93	0.94	0.94	0.97	1.04	0.96
U1-Essex	1.09	1.06	1.03	66 .0	0.94	06.0	0.88	0.86	0.93	0.94	0.99	1.06	0.93
U1-Southeast	1.06	1.05	1.01	0.97	0.95	0.93	0.93	06.0	0.94	0.94	0.98	1.04	0.98
U1-West	1.19	1.14	1.09	0.95	0.92	0.89	0.89	0.86	0.91	0.95	0.97	1.07	0.84
U1-Worcester	1.02	1.04	0.97	0.94	0.93	0.91	0.95	0.91	0.93	0.92	0.95	1.10	0.88
U2	1.01	1.00	0.94	0.93	0.91	0.89	0.93	06.0	06.0	0.91	0.94	1.02	0.99
U3	1.06	1.03	0.98	0.94	0.93	0.91	0.95	0.91	0.92	0.93	0.97	1.00	0.98
U4-U7	1.01	1.00	0.95	0.92	0.88	0.86	0.92	0.91	0.92	0.94	0.99	1.04	0.99
Rec - East	1.04	1.16	1.12	0.98	0.92	0.88	0.77	0.81	0.94	1.02	1.08	1.12	0.99
Rec - West	1.30	1.23	1.32	1.18	0.95	0.82	0.70	0.69	0.97	0.96	1.16	1.15	0.98

Round off:

0-999 = 10 >1000 = 100

U = Urban R = Rural 1 - Interstate

2 - Freeway and Expressway

3 - Other Principal Arterial

4 - Minor Arterial

5 - Major Collector

6 - Minor Collector

7 - Local Road and Street

7014,7079,7080,7090,7091,7092,7093,7094,7095,7096,7097,7108 and 7178), Martha's Vineyard and Nantucket. Recreational - East Group - Cape Cod (all towns) including the town of Plymouth south of Route 3A (stations

1066,1067,1083,1084,1085,1086,1087,1088,1089,1090,1091,1092,1093,1094,1095,1096,1097,1098,1099,1100,1101,1102,1103,1104,1105,1106,1107,1108,1113,1114, Recreational - West Group - Continuous Stations 2 and 189 including stations 1116,2196,2197 and 2198.

39 Lamartine Street Worcester, MA

5/31/2020

39 Lamartine Street Worcester, MA

updated 5/1/2020

MassDOT Yearly Growth Rates

	for c	lata fro	m 2014	to 2018	3
Growth Factors					
Group	Grow 2014 to 2015	Grow 2015 to 2016	Grow 2016 to 2017	Grow 2017 to 2018	Grow 2018 to 2019
R1	0	0.023	0.004	0.018	0.016
R2	0.05	0.068	0.004	0.014	0.014
R3	-0.038	0.002	0.008	0.011	0.06
R4-7	-0.01	0.003	0.001	0.011	0.012
Rec – East		0.032	0.02	0.041	0.025
Rec – West		0.051	-0.008	0.029	0
U1-Boston	0.061	0.07	-0.003	0.012	0.006
U1-Essex	0.024	0.025	0.007	0.014	0.011
U1- Southeast	0.05	0.062	0.021	0.014	0
U1-West	0.03	-0.027	0.02	0.028	0.013
U1- Worcester	0.042	0.005	0.018	0.01	0.01
U2	0.04	0.048	0.008	0.01	0.02
U3	0.011	0.013	0.011	0.014	0.004
U4-7	0.023	0.062	0.017	0.003	-0.004

Average Cra (Base	ash Rates, per Million En Type ed upon crash information o	tering Vehicles, by Intersection queried on June 26, 2018)
Location	Signalized Intersections	Unsignalized Intersections
Statewide	0.78	0.57
District 1*	0.80*	0.44*
District 2	0.89	0.62
District 3	0.89	0.61
District 4	0.73	0.57
District 5	0.75	0.57
District 6	0.71	0.52
Location	Signaliz	red Intersections

* - District 1 should use Statewide Rates due to low sample total

Exhibit 3-8 Motor Vehicle Stopping Sight Distances

		Stop	ping Sight Di	istance (ft) by	Percent Gra	de (%)	
	2 		Downgrade			Upgrade	
Design Speed	0	3	6	9	3	6	9
20	115	116	120	126	109	107	104
25	155	158	165	173	147	143	140
30	200	205	215	227	200	184	179
35	250	257	271	287	237	229	222
40	305	315	333	354	289	278	269
45	360	378	400	427	344	331	320
50	425	446	474	507	405	388	375
55	495	520	553	593	469	450	433
60	570	598	638	686	538	515	495
65	645	682	728	785	612	584	561
70	730	771	825	891	690	658	631
75	820	866	927	1003	772	736	704

Source: A Policy on Geometric Design of Streets and Highways, AASHTO, Washington DC, 2004. Chapter 3 Elements of Design

MASSIHIGHWAY

2006 EDITION



Sight Triangle Legs: Case B - Stop Control on Cross Street

	Length of Sight Triangle Legs (feet)											
Major Street Design Speed (mph)	Minor Street for Vehicles Approaching From Right (A _R , feet)	Minor Street for Vehicles Approaching From Left (AL, feet)	Major Street For Left Turns (B, feet)	Major Street for Right Turns or Through (B, feet)								
15	32.5	20.5	170	145								
20	32.5	20.5	225	195								
25	32.5	20.5	280	240								
30	32.5	20.5	335	290								
35	32.5	20.5	390	335								
40	32.5	20.5	445	385								
45	32.5	20.5	500	430								
50	32.5	20.5	555	480								
55	32.5	20.5	610	530								
60	32.5	20.5	665	575								
65	32.5	20.5	720	625								
70	32.5	20.5	775	670								
75	32.5	20.5	830	720								

Sight triangle legs shown are for passenger car crossing or turning into a two-lane street, with grades (all approaches) 3 percent or less. For other grades and for other major street widths, recalculate using AASHTO Green Book formulas.

Source: A Policy on Geometric Design of Streets and Highways, AASHTO, Washington DC, 2004. Chapter 3 Elements of Design

Trip Generation

Mid-Rise Residential with Ground-Floor Commercial GFA (1-25k) (231)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: Dense Multi-Use Urban

Number of Studies: 14

Avg. Num. of Dwelling Units: 181

Directional Distribution: 39% entering, 61% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.20	0.02 - 0.44	0.12

Data Plot and Equation



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ite=

Mid-Rise Residential with Ground-Floor Commercial GFA (1-25k) (231)



Dense Multi-Use Urban and Center City Core 421





Dense Multi-Use Urban and Center City Core 929

65



930 Trip Generation Manual 11th Edition • Volume 2

Multifamily Housing - 2+ BR (Mid-Rise) Not Close to Rail Transit (221)

Peak Period Parking Demand vs: Dwelling Units

On a: Weekday (Monday - Friday)

Setting/Location: Dense Multi-Use Urban

Number of Studies: 44

Avg. Num. of Dwelling Units: 120

Period Parking Demand per Dwelling Unit

⊷ær∋ge Rate	Range of Rates	33rd / 85th Percentile	95% Confidence Interval	Standard Deviation (Coeff. of Variation)
2.93	0.17 - 2.72	0.68 / 1.29	0.83 - 1.03	0.33 (35%)

Pot and Equation



Land Use Descriptions and Data Plots 121

	-+	>	-	-	-	-	
All second s			•		•		
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ef 👔			र्स	Y		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	70	5	3	103	14	9	
Peak Hour Factor	0.88	0.63	0.36	0.89	0.54	0.50	
Hourly flow rate (veh/h)	80	8	8	116	26	18	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			87		216	84	
vC1, stage 1 conf vol			2000 A			1998-199	
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		97	98	
cM capacity (veh/h)			1508		768	976	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	87	124	44				
Volume Left	0	8	26				
Volume Right	8	0	18				
cSH	1700	1508	841				
Volume to Capacity	0.05	0.01	0.05				
Queue Length (ft)	0	0	4				
Control Delay (s)	0.0	0.5	9.5				
Lane LOS		А	А				
Approach Delay (s)	0.0	0.5	9.5				
Approach LOS			А				
Intersection Summary							
Average Delay			1.9				
Intersection Capacity Uti	lization		17.1%	10	CU Leve	el of Servic	
				I.			ĺ

Intersection of Lamartine St & Meade St AM peak Existing Conditions

				A			A 1111
Intercection of	amartino	St X.	Maada	St PM	noak -	Victing	Conditions
Intersection of	Lamartine	ora	MCauc		pear L	Albung	Conditions

	-	7	*	-	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ef (÷.	Y		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	101	5	10	121	6	3	
Peak Hour Factor	0.77	0.63	0.75	0.94	0.34	0.42	
Hourly flow rate (veh/h)	131	8	13	129	18	7	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			139		291	135	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		97	99	
cM capacity (veh/h)			1444		694	914	
Direction, Lane #	EB 1	WB1	NB 1				
Volume Total	139	142	25				
Volume Left	0	13	18				
Volume Right	8	0	7				
cSH	1700	1444	745				
Volume to Capacity	0.08	0.01	0.03				
Queue Length (ft)	0	1	3				
Control Delay (s)	0.0	0.8	10.0				
Lane LOS		A	A				
Approach Delay (s)	0.0	0.8	10.0				
Approach LOS			A				
Intersection Summary							
Average Delay			1.2				
Intersection Capacity Uti	lization		18.7%	10	CU Leve	el of Servic	ce A

	→	7	4	-	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ħ			ų	Y		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	70	0	5	113	6	4	
Peak Hour Factor	0.92	0.25	0.42	0.86	0.75	0.50	
Hourly flow rate (veh/h)	76	0	12	131	8	8	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			76		231	76	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		99	99	
cM capacity (veh/h)			1523		751	985	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	76	143	16				
Volume Left	0	12	8				
Volume Right	0	0	8				
cSH	1700	1523	852				
Volume to Capacity	0.04	0.01	0.02				
Queue Length (ft)	0	1	1				
Control Delay (s)	0.0	0.7	9.3				
Lane LOS		А	А				
Approach Delay (s)	0.0	0.7	9.3				
Approach LOS			А				
Intersection Summary							
Average Delay			1.0				
Intersection Capacity Uti	lization		18.6%	10	CU Leve	el of Servi	i

Intersection of Lamartine St & Grosvenor St AM peak Existing Conditions

	-	7	•	←	1	1		
Movement	EBT	EBR	WBL	WBT	NBL	NBR		
Lane Configurations	f,			र्भ	Y			
Sign Control	Free			Free	Stop			
Grade	0%			0%	0%			
Volume (veh/h)	105	1	2	125	8	2		
Peak Hour Factor	0.78	0.25	0.50	0.85	0.63	0.50		
Hourly flow rate (veh/h)	135	4	4	147	13	4		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type					None			
Median storage veh)								
vC, conflicting volume			139		292	137		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
tC, single (s)			4.1		6.4	6.2		
tC, 2 stage (s)								
tF (s)			2.2		3.5	3.3		
p0 queue free %			100		98	100		
cM capacity (veh/h)			1445		697	912		
Direction, Lane #	EB 1	WB 1	NB 1					
Volume Total	139	151	17					
Volume Left	0	4	13					
Volume Right	4	0	4					
cSH	1700	1445	739					
Volume to Capacity	0.08	0.00	0.02					
Queue Length (ft)	0	0	2					
Control Delay (s)	0.0	0.2	10.0					
Lane LOS		А	А					
Approach Delay (s)	0.0	0.2	10.0					
Approach LOS			А					
Intersection Summary								
Average Delay			0.7					
Intersection Capacity Uti	lization		18.3%	10	CU Leve	el of Servio	се	

Intersection of Lamartine St & Grosvenor St PM peak Existing Conditions

Intersection of Lafa	yette St & Meade St AM	A Peak Existing Conditions
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	٠	-	-	*	1	-		
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		र्भ	4		Y			
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Volume (veh/h)	6	37	0	0	3	0		
Peak Hour Factor	0.85	0.80	0.92	0.92	0.33	0.33		
Hourly flow rate (veh/h)	7	46	0	0	9	0		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type					None			
Median storage veh)								
vC, conflicting volume	0				60	0		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
tC, single (s)	4.1				6.4	6.2		
tC, 2 stage (s)								
tF (s)	2.2				3.5	3.3		
p0 queue free %	100				99	100		
cM capacity (veh/h)	1623				942	1085		
Direction, Lane #	EB 1	WB 1	SB 1					
Volume Total	53	0	9					
Volume Left	7	0	9					
Volume Right	0	0	0					
cSH	1623	1700	942					
Volume to Capacity	0.00	0.00	0.01					
Queue Length (ft)	0	0	1					
Control Delay (s)	1.0	0.0	8.9					
Lane LOS	A		A					
Approach Delay (s)	1.0	0.0	8.9					
Approach LOS			A					
Intersection Summary								
Average Delay			2.1					
Intersection Capacity Uti	lization		13.3%	IC	CU Leve	el of Serv	vice A	
intersection of Laray	Cile C		aue o		CarL	visuing C	Junions	
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	٠	-	+	*	4	4		
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		aî	ĥ		M			
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Volume (veh/h)	3	64	0	0	8	0		
Peak Hour Factor	0.75	0.76	0.92	0.92	0.50	0.33		
Hourly flow rate (veh/h)	4	84	0	0	16	0		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type					None			
Median storage veh)								
vC, conflicting volume	0				92	0		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
tC, single (s)	4.1				6.4	6.2		
tC, 2 stage (s)								
tF (s)	2.2				3.5	3.3		
p0 queue free %	100				98	100		
cM capacity (veh/h)	1623				906	1085		
Direction, Lane #	EB 1	WB 1	SB 1					
Volume Total	88	0	16					
Volume Left	4	0	16					
Volume Right	0	0	0					
cSH	1623	1700	906					
Volume to Capacity	0.00	0.00	0.02					
Queue Length (ft)	0	0	1					
Control Delay (s)	0.3	0.0	9.0					
Lane LOS	А		А					
Approach Delay (s)	0.3	0.0	9.0					
Approach LOS			А					
Intersection Summary								
Average Delay			1.7					
Intersection Capacity Uti	ilization		14.7%		CU Leve	el of Servic	е	А

Intersection of Lafayette St & Meade St PM Peak Existing Conditions

Intersection of Lafayette St & Grosvenor St AM	Peak Existing Conditions
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	٠	-	+	•	5	1		
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		é.	f		Y			
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Volume (veh/h)	9	39	0	0	5	0		
Peak Hour Factor	0.75	0.75	0.92	0.92	0.62	0.33		
Hourly flow rate (veh/h) 12	52	0	0	8	0		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type					None			
Median storage veh)								
vC, conflicting volume	0				76	0		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
tC, single (s)	4.1				6.4	6.2		
tC, 2 stage (s)								
tF (s)	2.2				3.5	3.3		
p0 queue free %	99				99	100		
cM capacity (veh/h)	1623				920	1085		
Direction, Lane #	EB 1	WB1	SB 1					
Volume Total	64	0	8					
Volume Left	12	0	8					
Volume Right	0	0	0					
cSH	1623	1700	920					
Volume to Capacity	0.01	0.00	0.01					
Queue Length (ft)	1	0	1					
Control Delay (s)	1.4	0.0	8.9					
Lane LOS	А		А					
Approach Delay (s)	1.4	0.0	8.9					
Approach LOS			А					
Intersection Summary								
Average Delay			2.2					
Intersection Capacity L	Jtilization		13.4%	10	CU Leve	el of Servic	Э	

Intersection of Lafayette	St &	Grosvenor	St	PM	Peak	Existing	Conditions
•		-			1	1	

	0.00 0						
	٠	-	+	*	1	1	
10 J							
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ર્સ	4		Y		
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Volume (veh/h)	8	61	0	0	4	0	
Peak Hour Factor	0.50	0.80	0.92	0.92	0.50	0.33	
Hourly flow rate (veh/h)	16	76	0	0	8	0	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume	0				108	0	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)	4.1				6.4	6.2	
tC. 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	99				99	100	
cM capacity (veh/h)	1623				880	1085	
					2.5.5		
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	92	0	8				
Volume Left	16	0	8				
Volume Right	0	0	0				
cSH	1623	1700	880				
Volume to Capacity	0.01	0.00	0.01				
Queue Length (ft)	1	0	1				
Control Delay (s)	1.3	0.0	9.1				
Lane LOS	А		А				
Approach Delay (s)	1.3	0.0	9.1				
Approach LOS			А				
Intersection Summary							
Average Delay			1.9				
Intersection Capacity Uti	lization		14.9%	IC	CU Leve	el of Service	Э

Intersection of Lana		or, men	mon c	i, Lou		oreen		I Cak		y con	unions	
	٠	-	¥	4	+	*	1	Ť	1	1	ţ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	27	80	39	12	58	8	6	55	58	6	33	8
Peak Hour Factor	0.87	0.87	0.87	0.93	0.93	0.93	0.85	0.85	0.85	0.82	0.82	0.82
Hourly flow rate (veh/h)	31	92	45	13	62	9	7	65	68	7	40	10
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	168	84	140	57								
Volume Left (vph)	31	13	7	7								
Volume Right (vph)	45	9	68	10								
Hadj (s)	-0.1	0.0	-0.2	0.0								
Departure Headway (s)	4.3	4.6	4.3	4.6								
Degree Utilization, x	0.20	0.11	0.17	0.07								
Capacity (veh/h)	797	575	798	747								
Control Delay (s)	8.4	8.1	8.1	7.9								
Approach Delay (s)	8.4	8.1	8.1	7.9								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.2									
HCM Level of Service			А									
Intersection Capacity Uti	lization		30.9%	10	CU Leve	el of Ser	vice		А			

Intersection of Lamartine St. Hermon St. Lodi St & Green St AM Peak Existing Conditions

Intersection of Lamartine St, Ggreen Island Blvd, Lodi and Hermon Sts PM Peak Existing Conditions

	٠	-	7	-	-	*	1	Ť	1	5	ŧ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	7	52	31	22	34	4	14	49	70	16	103	17
Peak Hour Factor	0.89	0.89	0.89	0.79	0.79	0.79	0.81	0.81	0.81	0.76	0.76	0.76
Hourly flow rate (veh/h)	8	58	35	28	43	5	17	60	86	21	136	22
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	101	76	164	179								
Volume Left (vph)	8	28	17	21								
Volume Right (vph)	35	5	86	22								
Hadj (s)	-0.2	0.1	-0.3	0.0								
Departure Headway (s)	4.6	4.6	4.2	4.5								
Degree Utilization, x	0.13	0.10	0.19	0.22								
Capacity (veh/h)	730	574	810	778								
Control Delay (s)	8.2	8.1	8.2	8.7								
Approach Delay (s)	8.2	8.1	8.2	8.7								
Approach LOS	А	А	А	A								
Intersection Summary												
Delay			8.4									
HCM Level of Service			А									
Intersection Capacity Uti	lization		25.7%	1	CU Leve	el of Ser	vice		А			

	-	7	•	+	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	Þ			र्भ	Y		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	77	6	3	113	15	10	
Peak Hour Factor	0.88	0.63	0.36	0.89	0.54	0.50	
Hourly flow rate (veh/h)	88	10	8	127	28	20	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			97		236	92	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		96	98	
cM capacity (veh/h)			1496		748	965	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	97	135	48				
Volume Left	0	8	28				
Volume Right	10	0	20				
cSH	1700	1496	826				
Volume to Capacity	0.06	0.01	0.06				
Queue Length (ft)	0	0	5				
Control Delay (s)	0.0	0.5	9.6				
Lane LOS		А	А				
Approach Delay (s)	0.0	0.5	9.6				
Approach LOS			А				
Intersection Summary							
Average Delay			1.9				
Intersection Capacity Uti	lization		17.8%	10	CU Leve	el of Servic	ce

Intersection of Lamartine St & Meade St AM peak Future No Build Conditions

	-	7	4	+	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	Þ			र्भ	¥		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	111	6	11	138	7	3	
Peak Hour Factor	0.77	0.63	0.75	0.94	0.34	0.42	
Hourly flow rate (veh/h)	144	10	15	147	21	7	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			154		325	149	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		97	99	
cM capacity (veh/h)			1427		662	898	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	154	161	28				
Volume Left	0	15	21				
Volume Right	10	0	7				
cSH	1700	1427	710				
Volume to Capacity	0.09	0.01	0.04				
Queue Length (ft)	0	1	3				
Control Delay (s)	0.0	0.8	10.3				
Lane LOS		А	В				
Approach Delay (s)	0.0	0.8	10.3				
Approach LOS			В				
Intersection Summary							
Average Delay			1.2				
Intersection Canacity Litil							

Intersection of Lamartine St & Meade St PM peak Future No Build Conditions

	-	7	4	+	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4			4	Y		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	77	0	6	124	8	4	
Peak Hour Factor	0.92	0.25	0.42	0.86	0.75	0.50	
Hourly flow rate (veh/h)	84	0	14	144	11	8	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			84		256	84	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		99	99	
cM capacity (veh/h)			1513		725	976	
Direction, Lane #	EB 1	WB1	NB 1				
Volume Total	84	158	19				
Volume Left	0	14	11				
Volume Right	0	0	8				
cSH	1700	1513	815				
Volume to Capacity	0.05	0.01	0.02				
Queue Length (ft)	0	1	2				
Control Delay (s)	0.0	0.7	9.5				
Lane LOS		А	А				
Approach Delay (s)	0.0	0.7	9.5				
Approach LOS			А				
Intersection Summary							
Average Delay			1.1				
Intersection Capacity Uti	lization		20.0%	10	CU Leve	l of Serv	vi

Intersection of Lamartine St & Grosvenor St AM peak Future No Build Conditions

	-	7	4	←	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ħ			र्स	Y		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	116	1	2	138	10	3	
Peak Hour Factor	0.78	0.25	0.50	0.85	0.63	0.50	
Hourly flow rate (veh/h)	149	4	4	162	16	6	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			153		321	151	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		98	99	
cM capacity (veh/h)			1428		671	896	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	153	166	22				
Volume Left	0	4	16				
Volume Right	4	0	6				
cSH	1700	1428	720				
Volume to Capacity	0.09	0.00	0.03				
Queue Length (ft)	0	0	2				
Control Delay (s)	0.0	0.2	10.2				
Lane LOS		А	В				
Approach Delay (s)	0.0	0.2	10.2				
Approach LOS			В				
Intersection Summary							
Average Delay			0.8				
Intersection Capacity Uti	lization		19.2%	10	CU Leve	el of Serv	vio
1							

Intersection of Lamartine St & Grosvenor St PM peak Future No Build Conditions

Intersection of La	afayette St & I	<mark>Mead</mark> e St	AM	Peak F	uture	No Build	Conditions
	*	-		1	1		

	٦	→	←	*	1	1		
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		ŧ	f)		Y			
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Volume (veh/h)	7	41	0	0	3	0		
Peak Hour Factor	0.85	0.80	0.92	0.92	0.33	0.33		
Hourly flow rate (veh/h) 8	51	0	0	9	0		
Pedestrians	10							
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type					None			
Median storage veh)								
vC, conflicting volume	0				68	0		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
tC, single (s)	4.1				6.4	6.2		
tC, 2 stage (s)								
tF (s)	2.2				3.5	3.3		
p0 queue free %	99				99	100		
cM capacity (veh/h)	1623				932	1085		
Direction, Lane #	EB 1	WB 1	SB 1					
Volume Total	59	0	9					
Volume Left	8	0	9					
Volume Right	0	0	0					
cSH	1623	1700	932					
Volume to Capacity	0.01	0.00	0.01					
Queue Length (ft)	0	0	1					
Control Delay (s)	1.0	0.0	8.9					
Lane LOS	A		А					
Approach Delay (s)	1.0	0.0	8.9					
Approach LOS			А					
Intersection Summary								
Average Delay			2.1					
Intersection Capacity I	Jtilization		13.3%	10	CU Leve	el of Servic	е	

Intersection	of Lafayette	St &	Meade	St PM	Peak	Future	No Build	Conditions
	٨		. +	- 4	1	1		

	٠	→	+	*	1	1	
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		ŧ	ħ		¥		
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Volume (veh/h)	3	70	0	0	9	0	
Peak Hour Factor	0.75	0.76	0.92	0.92	0.50	0.33	
Hourly flow rate (veh/h)	4	92	0	0	18	0	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume	0				100	0	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	100				98	100	
cM capacity (veh/h)	1623				896	1085	
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	96	0	18				
Volume Left	4	0	18				
Volume Right	0	0	0				
cSH	1623	1700	896				
Volume to Capacity	0.00	0.00	0.02				
Queue Length (ft)	0	0	2				
Control Delay (s)	0.3	0.0	9.1				
Lane LOS	А		A				
Approach Delay (s)	0.3	0.0	9.1				
Approach LOS			A				
Intersection Summary							
Average Delay			1.7				
Intersection Capacity Uti	lization		15.1%	IC	CU Leve	el of Service	e A

Intersection of Lafayette St & Grosvenor St AM Peak Future No Build Conditions

	٠	_	+	*	1	1	
Movement	FRI	FRT	WRT	W/BR	SBI	SBR	
Lane Configurations	LDL		1.	WDIX	UDL M	ODIX	
Sign Control		Free	Free		Stop		
Grade		0%	0%		0%		
Volume (veh/h)	10	13	0 /0	0	6,0	٥	
Peak Hour Factor	0.75	0.75	0.92	0.92	0.62	0.33	
Hourly flow rate (yeb/k	0.75	57	0.32	0.52	10	0.00	
Pedestrians	1) 13	51	0	0	10	0	
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)					NUTIC		
vC conflicting volume	٥				84	0	
vC1_stage 1 conf vol	U				04	U	
vC2 stage 2 conf vol							
tC. single (s)	4 1				64	62	
tC, 2 stage (s)	1.1				0.1	0.2	
tE (s)	22				35	3.3	
n0 queue free %	99				99	100	
cM capacity (veh/h)	1623				910	1085	
	1020		The second second		010	1000	
Direction, Lane #	EB 1	WB1	SB 1				
Volume Total	71	0	10				
Volume Left	13	0	10				
Volume Right	0	0	0				
cSH	1623	1700	910				
Volume to Capacity	0.01	0.00	0.01				
Queue Length (ft)	1	0	1				
Control Delay (s)	1.4	0.0	9.0				
Lane LOS	A		А				
Approach Delay (s)	1.4	0.0	9.0				
Approach LOS			A				
Intersection Summary							
Average Delay			2.3				
Intersection Capacity	Utilization		13.8%	IC	CU Leve	el of Servio	ce A

Intersection of Lafav	ette St &	Grosvenor	St PM	Peak	Euture No	Build	Conditions
intersection of Eulay	chie of a		OUTIV	i cun	i diule i io	Dunu	Conditionio
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N 4	EDI	EDT	MOT		0.01	000	
Novement	EBL	ERI	VVBI	WBK	SBL	SBR	
Lane Configurations		_ ଶି	- T+		Y		
Sign Control		Free	Free		Stop		
Grade	-	0%	0%	-	0%		
Volume (veh/h)	9	67	0	0	4	0	
Peak Hour Factor	0.50	0.80	0.92	0.92	0.50	0.33	
Hourly flow rate (veh/h)	18	84	0	0	8	0	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)	_						
vC, conflicting volume	0				120	0	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	99				99	100	
cM capacity (veh/h)	1623				866	1085	
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	102	0	8				
Volume Left	18	0	8				
Volume Right	0	0	0				
cSH	1623	1700	866				
Volume to Capacity	0.01	0.00	0.01				
Queue Length (ft)	1	0	1				
Control Delay (s)	1.4	0.0	9.2				
Lane LOS	А		А				
Approach Delay (s)	1.4	0.0	9.2				
Approach LOS			А				
Intersection Summary							
Average Delay			1.9				
Intersection Capacity Uti	lization		15.4%	IC	CU Leve	el of Servio	ce A

	٠	-+	7	1	-	*	1	Ť	1	1	ŧ	~
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	30	88	43	14	64	9	7	61	65	7	37	9
Peak Hour Factor	0.87	0.87	0.87	0.93	0.93	0.93	0.85	0.85	0.85	0.82	0.82	0.82
Hourly flow rate (veh/h)	34	101	49	15	69	10	8	72	76	9	45	11
Direction, Lane #	EB 1	WB1	NB 1	SB 1								
Volume Total (vph)	185	94	156	65								
Volume Left (vph)	34	15	8	9								
Volume Right (vph)	49	10	76	11								
Hadj (s)	-0.1	0.0	-0.2	0.0								
Departure Headway (s)	4.4	4.7	4.3	4.7								
Degree Utilization, x	0.23	0.12	0.19	0.08								
Capacity (veh/h)	772	568	781	730								
Control Delay (s)	8.7	8.3	8.4	8.1								
Approach Delay (s)	8.7	8.3	8.4	8.1								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.4									
HCM Level of Service			А									
Intersection Capacity Uti	lization		33.4%	1	CU Lev	el of Ser	vice		А			

Intersection of Lamartine St, Hermon St, Lodi St & Green St AM Peak Future No Build Condition

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			\$			\$	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	8	57	34	24	37	4	15	54	78	18	114	19
Peak Hour Factor	0.89	0.89	0.89	0.79	0.79	0.79	0.81	0.81	0.81	0.76	0.76	0.76
Hourly flow rate (veh/h)	9	64	38	30	47	5	19	67	96	24	150	25
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	111	82	181	199								
Volume Left (vph)	9	30	19	24								
Volume Right (vph)	38	5	96	25								
Hadj (s)	-0.2	0.1	-0.3	0.0								
Departure Headway (s)	4.7	4.7	4.3	4.5								
Degree Utilization, x	0.14	0.11	0.22	0.25								
Capacity (veh/h)	712	568	795	765								
Control Delay (s)	8.5	8.2	8.5	9.0								
Approach Delay (s)	8.5	8.2	8.5	9.0								
Approach LOS	А	А	А	A								
Intersection Summary												
Delay			8.6									
HCM Level of Service			А									
Intersection Capacity Uti	lization		28.9%	10	CU Leve	el of Ser	vice		А			

Intersection of Lamartine St, Ggreen Island Blvd, Lodi and Hermon Sts PM Peak Future No Build Conditions

	→	¥	4	+	1	1			
Movement	EBT	EBR	WBL	WBT	NBL	NBR			
Lane Configurations	4			र्स	Y				
Sign Control	Free			Free	Stop				
Grade	0%			0%	0%				
Volume (veh/h)	77	6	4	113	15	11			
Peak Hour Factor	0.88	0.63	0.36	0.89	0.54	0.50			
Hourly flow rate (veh/h)	88	10	11	127	28	22			
Pedestrians									
Lane Width (ft)									
Walking Speed (ft/s)									
Percent Blockage									
Right turn flare (veh)									
Median type					None				
Median storage veh)									
vC, conflicting volume			97		241	92			
vC1, stage 1 conf vol									
vC2, stage 2 conf vol									
tC, single (s)			4.1		6.4	6.2			
tC, 2 stage (s)									
tF (s)			2.2		3.5	3.3			
p0 queue free %			99		96	98			
cM capacity (veh/h)			1496		741	965			
Direction, Lane #	EB 1	WB 1	NB 1						
Volume Total	97	138	50						
Volume Left	0	11	28						
Volume Right	10	0	22						
cSH	1700	1496	826						
Volume to Capacity	0.06	0.01	0.06						
Queue Length (ft)	0	1	5						
Control Delay (s)	0.0	0.7	9.6						
Lane LOS		А	А						
Approach Delay (s)	0.0	0.7	9.6						
Approach LOS			А						
Intersection Summary									
Average Delay			2.0						
Intersection Capacity Uti	lization		18.2%	IC	CU Leve	el of Serv	i	се	се
1						ver det offerfille fe			

Intersection of Lamartine St & Meade St AM peak Future Build Conditions

	→	>	1	+	•	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1			4	M		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	111	6	12	138	8	3	
Peak Hour Factor	0.77	0.63	0.75	0.94	0.34	0.42	
Hourly flow rate (veh/h	n) 144	10	16	147	24	7	
Pedestrians	.,						
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume)		154		328	149	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		96	99	
cM capacity (veh/h)			1427		659	898	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	154	163	31				
Volume Left	0	16	24				
Volume Right	10	0	7				
cSH	1700	1427	703				
Volume to Capacity	0.09	0.01	0.04				
Queue Length (ft)	0	1	3				
Control Delay (s)	0.0	0.8	10.4				
Lane LOS		A	В				
Approach Delay (s)	0.0	0.8	10.4				
Approach LOS			В				
Intersection Summary	1						
Average Delay			1.3				
Intersection Capacity	Utilization		20.5%	10	CU Leve	el of Service	e

Intersection of Lamartine St & Meade St PM peak Future Build Conditions

	→	7	1	+	1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	1.			र्भ	Y		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	77	1	6	124	9	5	
Peak Hour Factor	0.92	0.25	0.42	0.86	0.75	0.50	
Hourly flow rate (veh/h)	84	4	14	144	12	10	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			88		258	86	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			99		98	99	
cM capacity (veh/h)			1508		723	973	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	88	158	22				
Volume Left	0	14	12				
Volume Right	4	0	10				
cSH	1700	1508	819				
Volume to Capacity	0.05	0.01	0.03				
Queue Length (ft)	0	1	2				
Control Delay (s)	0.0	0.7	9.5				
Lane LOS		А	А				
Approach Delay (s)	0.0	0.7	9.5				
Approach LOS			А				
Intersection Summary							
Average Delay			1.2				
Intersection Capacity Util	ization		20.0%	10	CU Leve	el of Servic	

Intersection of Lamartine St & Grosvenor St AM peak Future Build Conditions

					100	1213	
	-	7	1		1	1	
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ħ			र्स	Y		
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Volume (veh/h)	116	3	2	138	12	4	
Peak Hour Factor	0.78	0.25	0.50	0.85	0.63	0.50	
Hourly flow rate (veh/h)	149	12	4	162	19	8	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume			161		325	155	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)			4.1		6.4	6.2	
tC, 2 stage (s)							
tF (s)			2.2		3.5	3.3	
p0 queue free %			100		97	99	
cM capacity (veh/h)			1418		667	891	
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	161	166	27				
Volume Left	0	4	19				
Volume Right	12	0	8				
cSH	1700	1418	721				
Volume to Capacity	0.09	0.00	0.04				
Queue Length (ft)	0	0	3				
Control Delay (s)	0.0	0.2	10.2				
Lane LOS		А	В				
Approach Delay (s)	0.0	0.2	10.2				
Approach LOS			В				
Intersection Summary							
Average Delay			0.9				
Intersection Capacity Uti	lization		19.2%	IC	CU Leve	el of Servi	ce
1							

Intersection of Lamartine St & Grosvenor St PM peak Future Build Conditions

Intersection of Lafayette St & Meade St AM Peak Future Build Condition	5
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Intersection of Larayette St & Meade St AM Feak Future Dully Conditions											
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Movement	EBL	EBT	WBT	WBR	SBL	SBR					
Lane Configurations		र्स	ĥ		Y						
Sign Control		Free	Free		Stop						
Grade		0%	0%		0%						
Volume (veh/h)	8	41	0	0	3	0					
Peak Hour Factor	0.85	0.80	0.92	0.92	0.33	0.33					
Hourly flow rate (veh/h)	9	51	0	0	9	0					
Pedestrians											
Lane Width (ft)											
Walking Speed (ft/s)											
Percent Blockage											
Right turn flare (veh)											
Median type					None						
Median storage veh)											
vC, conflicting volume	0				70	0					
vC1, stage 1 conf vol											
vC2, stage 2 conf vol											
tC, single (s)	4.1				6.4	6.2					
tC, 2 stage (s)											
tF (s)	2.2				3.5	3.3					
p0 queue free %	99				99	100					
cM capacity (veh/h)	1623				929	1085					
Direction, Lane #	EB 1	WB 1	SB 1								
Volume Total	61	0	9								
Volume Left	9	0	9								
Volume Right	0	0	0								
cSH	1623	1700	929								
Volume to Capacity	0.01	0.00	0.01								
Queue Length (ft)	0	0	1								
Control Delay (s)	1.2	0.0	8.9								
Lane LOS	А		А								
Approach Delay (s)	1.2	0.0	8.9								
Approach LOS			А								
Intersection Summary											
Average Delay			2.2								
Intersection Capacity Uti	lization		13.3%	IC	CU Leve	el of Service	е	А			

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Intersection	of Lafayette	St &	Meade	St PM	Peak	Future	Build	Conditio	ons
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Movement	EBI	FBT	WBT	WBR	SBI	SBR	
	LDL			VUDIN	ODL	ODK	
Sign Control		Froo	Froo		Stop		
Grado		0%	CIEC 004		Stop		
Volume (voh/h)	٨	70	0%	0	10	0	
Volume (ven/n)	0.75	70	0.00	0 00	10	0 22	
Peak Hour Factor	0.75	0.76	0.92	0.92	0.50	0.33	
Houriy flow rate (ven/n)	5	92	0	0	20	0	
Lane width (π)							
waiking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type					None		
Median storage veh)							
vC, conflicting volume	0				103	0	
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
tC, single (s)	4.1				6.4	6.2	
tC, 2 stage (s)							
tF (s)	2.2				3.5	3.3	
p0 queue free %	100				98	100	
cM capacity (veh/h)	1623				892	1085	
Direction, Lane #	EB 1	WB 1	SB 1				
Volume Total	97	0	20				
Volume Left	5	0	20				
Volume Right	0	0	0				
cSH	1623	1700	892				
Volume to Capacity	0.00	0.00	0.02				
Queue Length (ft)	0	0	2				
Control Delay (s)	0.4	0.0	91				
Lane LOS	Δ	0.0	Δ				
Approach Delay (s)	0.4	0.0	91				
Approach LOS	0.4	0.0	Δ				
			7				
Intersection Summary							
Average Delay			1.9				
Intersection Capacity Uti	lization		15.1%	IC	CU Leve	el of Serv	lice

Intersection	of Lafayette	St &	Grosvenor	St AM	Peak	Future	Build	Conditio	ons
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Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		د	f)		Y			
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Volume (veh/h)	11	43	0	0	7	0		
Peak Hour Factor	0.75	0.75	0.92	0.92	0.62	0.33		
Hourly flow rate (veh/h)	15	57	0	0	11	0		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type					None			
Median storage veh)								
vC, conflicting volume	0				87	0		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
tC, single (s)	4.1				6.4	6.2		
tC, 2 stage (s)								
tF (s)	2.2				3.5	3.3		
p0 queue free %	99				99	100		
cM capacity (veh/h)	1623				906	1085		
Direction, Lane #	EB 1	WB 1	SB 1					
Volume Total	72	0	11					
Volume Left	15	0	11					
Volume Right	0	0	0					
cSH	1623	1700	906					
Volume to Capacity	0.01	0.00	0.01					
Queue Length (ft)	1	0	1					
Control Delay (s)	1.5	0.0	9.0					
Lane LOS	А		А					
Approach Delay (s)	1.5	0.0	9.0					
Approach LOS			А					
Intersection Summary								
Average Delay			2.5					
Intersection Capacity Uti	ilization		13.8%	10	CU Leve	el of Serv	ice	

Intersection of Lafa	yette St & Grosv	enor St PM Peak	Future Build	Conditions

	٠	-	+	~	1	-		
Movement	EBL	EBT	WBT	WBR	SBL	SBR		
Lane Configurations		र्स	Ţ.		¥			
Sign Control		Free	Free		Stop			
Grade		0%	0%		0%			
Volume (veh/h)	10	67	0	0	6	0		
Peak Hour Factor	0.50	0.80	0.92	0.92	0.50	0.33		
Hourly flow rate (veh/h	n) 20	84	0	0	12	0		
Pedestrians								
Lane Width (ft)								
Walking Speed (ft/s)								
Percent Blockage								
Right turn flare (veh)								
Median type					None			
Median storage veh)								
vC, conflicting volume	0				124	0		
vC1, stage 1 conf vol								
vC2, stage 2 conf vol								
tC, single (s)	4.1				6.4	6.2		
tC, 2 stage (s)								
tF (s)	2.2				3.5	3.3		
p0 queue free %	99				99	100		
cM capacity (veh/h)	1623				861	1085		
Direction, Lane #	EB 1	WB1	SB 1					
Volume Total	104	0	12					
Volume Left	20	0	12					
Volume Right	0	0	0					
cSH	1623	1700	861					
Volume to Capacity	0.01	0.00	0.01					
Queue Length (ft)	1	0	1					
Control Delay (s)	1.5	0.0	9.2					
Lane LOS	A		А					
Approach Delay (s)	1.5	0.0	9.2					
Approach LOS			A					
Intersection Summary	6							
Average Delay			2.3					
Intersection Capacity	Utilization		15.5%	10	CU Leve	el of Service	А	

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			\$			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	30	88	43	15	64	9	7	61	66	7	37	9
Peak Hour Factor	0.87	0.87	0.87	0.93	0.93	0.93	0.85	0.85	0.85	0.82	0.82	0.82
Hourly flow rate (veh/h)	34	101	49	16	69	10	8	72	78	9	45	11
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	185	95	158	65								
Volume Left (vph)	34	16	8	9								
Volume Right (vph)	49	10	78	11								
Hadj (s)	-0.1	0.0	-0.3	0.0								
Departure Headway (s)	4.4	4.7	4.3	4.7								
Degree Utilization, x	0.23	0.12	0.19	0.08								
Capacity (veh/h)	771	567	781	729								
Control Delay (s)	8.7	8.3	8.4	8.1								
Approach Delay (s)	8.7	8.3	8.4	8.1								
Approach LOS	A	А	А	А								
Intersection Summary												
Delay			8.4									
HCM Level of Service			А									
Intersection Capacity Uti	lization		33.5%	10	CU Leve	el of Ser	vice		А			

Intersection of Lamartine St, Hermon St, Lodi St & Green St AM Peak Future Build Conditions

Intersection of Lamartine St, Ggreen Island Blvd, Lodi and Hermon Sts PM Peak Future Build Conditions

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			4			4			4	
Sign Control		Stop			Stop			Stop			Stop	
Volume (veh/h)	8	57	35	25	37	4	16	54	80	18	114	19
Peak Hour Factor	0.89	0.89	0.89	0.79	0.79	0.79	0.81	0.81	0.81	0.76	0.76	0.76
Hourly flow rate (veh/h)	9	64	39	32	47	5	20	67	99	24	150	25
Direction, Lane #	EB 1	WB1	NB 1	SB 1								
Volume Total (vph)	112	84	185	199								
Volume Left (vph)	9	32	20	24								
Volume Right (vph)	39	5	99	25								
Hadj (s)	-0.2	0.1	-0.3	0.0								
Departure Headway (s)	4.7	4.7	4.3	4.5								
Degree Utilization, x	0.15	0.11	0.22	0.25								
Capacity (veh/h)	711	566	794	763								
Control Delay (s)	8.5	8.3	8.5	9.0								
Approach Delay (s)	8.5	8.3	8.5	9.0								
Approach LOS	А	А	А	А								
Intersection Summary												
Delay			8.7									
HCM Level of Service			А									
Intersection Capacity Uti	lization		29.0%	1	CU Leve	el of Ser	vice		А			